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Progress Report

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A SINEGLAR BASIS FOR TRAVEL PARTHERN STRIVEYS

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Gordon A. Shank Graduate Instructor in Research

Joint Rightmy Decearch Project

Project Mc. 0-36-69B

File No. 5 7 2

Propared as last of an investigation

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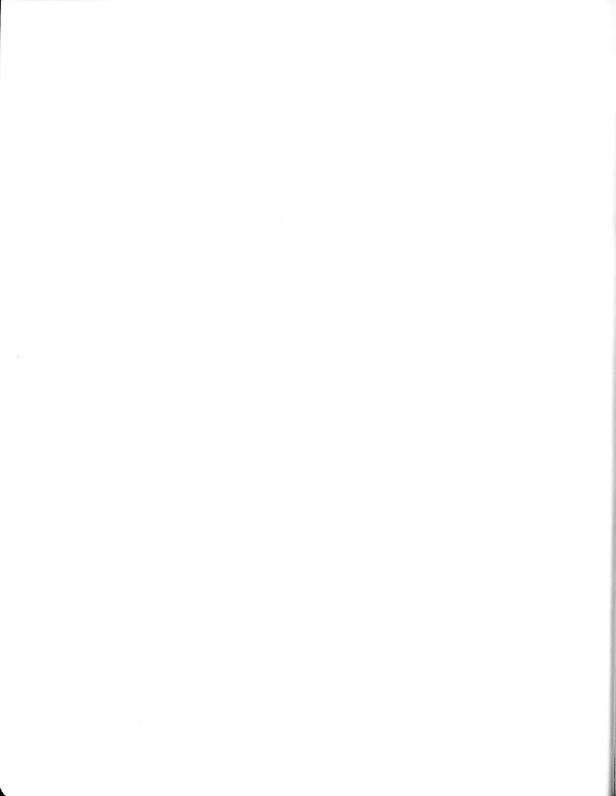
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ABSTRACT

Shunk, Gordon Arthur. Ph.D., Purdue University, January 1968. THE JOURNEY TO WORK: A SINGULAR BASIS FOR TRAVEL PATTERN SURVEYS. Major Professor: William L. Grecco.

This research involved a study of the feasibility of using the patterns of work trips alone to represent the patterns of travel for all purposes in an urban area. Further, the feasibility of using peak hour travel patterns to represent those of the entire day was investigated. The objective was to develop an approach to travel surveys which would satisfactorily reproduce the results of a conventional home interview survey.

Using data from a 1964 comprehensive transportation survey in Indianapolis, Indiana, an analysis of variance was run to determine the effect of the commonly defined factors, mode, purpose, and time, on trip volume and average trip length. Based on the extremely high significance of all main effects and interactions, a second variance analysis was run to determine the effect of more specific purpose, time, and mode factors on the traffic assigned to the freeway and arterial links of the highway system. The significance of all main effects and a mode-purpose interaction was the basis for regression models accounting for mode, the peak hour, and the work purpose. A high degree of the variation in total



trips on all major street system links was explained by multiple linear regression equations based on link volumes for the work purpose. Using the same regression approach, high degrees of explanation were achieved for total day, all purpose trips using all peak hour trips, and for all peak hour trips using total day work trips.

The results of this research recommend that travel patterns could be represented by surveys at the destination of trips. One of these would involve tabulation of the residence address from employer records. A second would obtain trip information by employee interviews. The third would, in addition, tabulate arrivals at major shopping areas. Decision on the form of the revised procedure would be on the basis of cost and feasibility. This research has provided for the study director alternate procedures for replacing the costly home-interview survey.

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CHAPTER I. URBAN TRANSPORTATION PLANNING

The current condition of intracity transportation systems poses one of the gravest problems faced by urban areas today. The very life of the city depends on the ease with which people and goods can move. The analogy to the human internal circulation system is quite valid. Urban transportation or circulation is highly complex in its own right. But considered in its proper perspective as an integral part of the total function of the urban body, the ramifications of intracity movement are nearly beyond conceivable complexity. Without effective means of moving the necessary nourishment between its organs, the urban complex will stagnate, wither or die. The solution of existing problems and prevention of their recurrence are the objectives of the activity known as urban transportation planning. Properly executed as a concordant phase of a comprehensive urban development program, the implementation of planned transportation improvements can be a major catalyst in the realization of broader social and economic objectives. In a time when the problems of urbanity are of such concern, the possibilities presented by transportation planning are of even more interest than usual (Turner, 1967).

In seeking a solution to any problem, it is well to reflect on just what led to development of the undesirable



situation. The first half of the twentieth century saw marked advances in the technology of nearly every field of endeavor. The advent of improved products and procedures was multiplicative, yielding more than singular results in advancement of the economy. The effects of the advancing economy were passed on to consumers through higher wages and more purchasing power. One of the products of the improved technology was very attractive to the newly affluent. The automobile would permit one to expand his choice of environment by traveling from his place of residence with ease and at relatively low cost. The automobile became a symbol of the affluence of the new economy and the society it fostered. With higher productivity came more leisure time in which to enjoy the benefits of affluence. Time became a more significant factor in everyday life, and means of saving time were important. The automobile permitted one to come and go when he wished and to travel the routes he desired.

There was little problem due to the automobile before World War II. As the economy and technology had advanced to where people could afford the large number of cars being produced, the depression retarded the trend in both phases. Automobile manufacturing during the war was curtailed because of the demand on resources for the defense effort. Following the war, the high level of the economy and increased production capacity combined to produce an explosion in auto sales. The romance with the automobile occurred so rapidly, in fact, that it would veritably defy prediction. The concomitant



increase in traffic volume came much too fast for street improvements to keep pace. By its very nature government is slow to adjust to dynamic situations. But the magnitude of the postwar auto boom left war oriented administrations nearly helpless. The possible problem had been suggested prior to the war (U. S. Congress, 1939) and had even been accounted for in policy as early as 1944 (U. S. Congress, 1944). These documents were primarily concerned with intercity movement. These still reflected an unawareness of the magnitude of the intracity transportation problem. When the auto boom occurred, there were not enough streets to handle the cars, and the traffic problem, as we know it today, had begun.

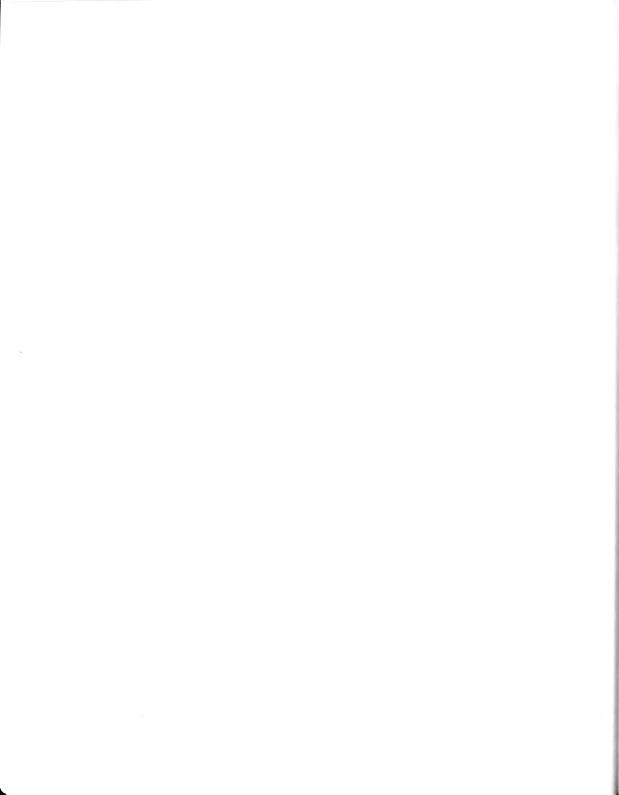
The traffic problem is not merely one of numbers. Considerably less trouble would occur if existing travel were spread uniformly over time. But the economies of scale that improved productivity also taught the logic of centralized activity which is feasible on a large scale only if adequate transportation is available. Centralized mass production requires simultaneous servicing, thereby demanding that all workers participate in their respective activities at the same time. This fosters the periodic high volumes of traffic which cause the greatest problems for transportation systems.

The magnitude of traffic volumes required new street designs to handle them. The most obvious remedy was to provide additional traffic handling capacity. From this was born the multilane facility and eventually divided expressways. But it was even difficult to design adequate expressways,



since the volume of traffic on them could not be predicted reliably. There were so many deficient streets and the deficiencies were so great that the normal lag time between design and construction increased. This effect was compounded by the greater than expected traffic increase over time. An additional factor causing trouble was the large volume of traffic using new facilities that had not traveled by car before, as well as that diverted from other facilities. All these situations contributed to the immediate overloading so often observed on new urban street facilities.

In 1952 congress launched the first overt attempt to accelerate construction of highways adequate to handle the huge traffic volumes (U. S. Congress, 1952). Following the Korean involvement, in 1956, Congress emphasized the importance of the Interstate System by increasing the rate of government participation in financing to 90 percent of the total cost (U. S. Congress, 1956). It had taken over ten years for Congress to gain sufficient realization of the gravity of the highway traffic problem to make a concerted effort toward its alleviation. The problems in this situation soon became apparent; perhaps it was the very magnitude of the Interstate program which brought out its shortcomings. The amount of money available for construction of the Interstate System permitted such programs as a 10 million dollar per mile freeway through the heart of an urban area. tions such as this awoke the public to the magnitude of their monster.



Although principally an intercity network, the Interstate System also contained substantial mileage within urban areas. These segments would be used extensively for intracity travel and would have to depend on urban traffic projections for design volume estimates. High quality facilities require much space and would displace considerable amounts of urban activity at commensurately high costs. It was an economic necessity to base design on future traffic volumes considerably more reliable than had previously been available.

Work had begun in the late 1940's to develop a more valid basis for highway design than mere factoring of existing traffic volumes. This work recognized the already apparent inability of projections to cope with the problem of forecasting urban traffic volumes. The new approach considered travel rather than traffic. It looked at traffic volume as merely a minor part of a much more complex universe of entire trips between an origin and a destination for a specific purpose. Travel, it was theorized, could be much more reliably predicted for a future time than could traffic, because there was an inherent basis for growth projection. It was the development of this approach and the realization of the need to plan for highways as an integral part of the community that led to the Federal Aid Highway Act of 1962 (U. S. Congress, 1962). This legislation requires that all urban highways must be designed as part of a comprehensive planning program. Such a program must account for land using activity



and its capability for generating trips. It must predict land use and thereby develop travel patterns for location of facilities and traffic volumes on which size is based. This new approach to traffic volume prediction is commonly termed Urban Transportation Planning (UTP).

The UTP approach to traffic volume prediction examines highways in their proper perspective as an integral part of urban activity. It provides not only volume or dimensional design data but locational information as well. It also considers non-highway modes of transport in order to attain balanced transportation systems. UTP provides more than a mere projection of current travel patterns; it permits modification of historic trends due to variations in development policies. The results of UTP are a much more valid and a firm basis for design of transportation facilities.

UTP is based on surveys of current and historic patterns of travel, development and the economy of the area. The elements of urban activity are isolated by thorough analysis of these survey data. The growth and development characteristics of those elements which influence travel are then established. The character of such elements is readily understood and predictable because of the traditional background of the elements as urban activity parameters. These elements then are projected or predicted to some future target date. Prediction here denotes variation from the results due to simple projection of trends. Based on the future levels of the chosen elements, future travel patterns



are developed for use in design. This procedure takes advantage of the reliable prediction of basic elements for which the growth patterns are known and stable. The relation between these basic elements and travel patterns in the survey year is used, in perhaps a modified form, to determine travel patterns in the target year. The results are based on underlying causal factors and are assumed to be more valid than simple volume projection.

The execution of the UTP process is quite intricate. Figure 1 shows a general diagram of the UTP procedure. It requires detailed study of the urban area and careful design of the respective surveys to account for all important local conditions. The surveys conducted are of varying depth and character. The economic base study surveys the current and historic condition of the local economy, its elements and interactions, strength and potential. The land use survey examines the qualitative and quantitative aspects of activity on every parcel of land in the urban area. It also includes analysis of the reasons for observed development patterns. Population studies determine the several important compositions of the local community. Other surveys may be of interest depending on the background and conditions of the local area.

The most detailed surveys are concerned with travel and the transportation system. The transportation system is inventoried in depth to establish the characteristics of the network of each available mode. The network is defined by



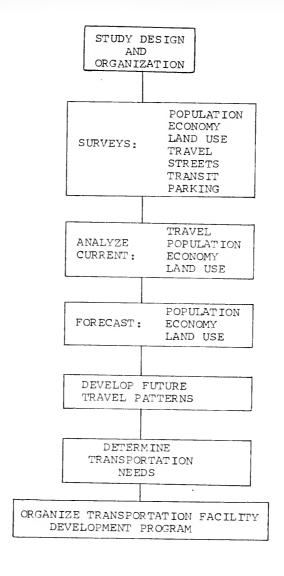


Figure 1. The Urban Transportation Planning Process.



a series of links describing each constituent of the system. Links are paths connecting nodes or junction points of the network. By using a series of links and nodes, any given route, i.e., transit route or street, may be represented in numeric form suitable for analysis. The entire network, be it highway or transit, is a compendium of interlinking and overlapping routes. Access to the network is via centroids located at activity centers termed origins or destinations. The network inventory not only establishes a link and node configuration, it also places on each link (or node) certain restrictions which define the character of that segment of the system (e.g., travel time, fare, capacity). Also obtained are existing volumes on each network link. The result is a mathematical description of the various components of the transportation system suitable for further analytical examination.

The survey of travel patterns involves considerable effort. Travel is surveyed in three phases, each of which examines only a sample of the universe of trips it represents. The trips made by trucks and taxis during an entire day are surveyed by examining the drivers' delivery records at the places where the vehicles are garaged. Truck surveys may also determine the type of goods carried. Trips into and out of the study area are examined by a "cordon" interview technique. Drivers of vehicles entering or leaving the area are queried as they cross a boundary which circumscribes the study area. Each interview concerns only the immediate trip



of the vehicle. The cordon survey is done in coordination with a volume counting program; expansion of the trip sample to total day travel is based on the volume count at the point of interview. The major phase of the travel pattern survey involves a personal interview at the dwelling unit to examine all trips made by all members of the respective household during an entire day. The home interview survey is probably the most expensive single activity in the entire UTP process. All travel pattern surveys obtain in some form information on the geographic origin and destination of trips and the purposes for which the trips were made.

The information obtained in the home-interview survey, and to a certain extent the other surveys, is organized by analysis zones. These zones are geographic entities developed in a manner to facilitate execution and expansion of the sample surveys. Trips are coded according to the zone numbers at their origins and destinations. Land use is grouped to indicate the activity in each zone; population studies are broken down to the zone level. It is the same zone that is represented by the centroid in the link-node description of the transportation system.

The data obtained from the several surveys are summarized for each of the analysis zones. The networks are processed to obtain the effective separation (time or distance) between all zone pairs. These data are used to calibrate a mathematical representation or model describing propensity to travel between zones for various trip purposes. Such a model



may take any of several forms and relates the reluctance to travel to the separation between the zones. The model is termed a distribution model for it describes the manner in which trips are arrayed over the area according to the attractions observed in various zones. Calibration of a distribution model involves defining the function of certain parameters in the equation for the current pattern of travel. These parameters describe the manner in which zonal separation affects the propensity to travel in the given urban area at the current time. Other parameters may also be evaluated. It is assumed that the observed relation will continue to exist or will change in a predictable manner to the target year. Thus the observed (or modified) parameters are used with predicted attraction values to determine travel patterns in the target year.

The elements affecting travel are then predicted for the target year. Population is projected in the standard manner; the activity of the local economy is projected in a manner which accounts for stimulative or restraining factors which are expected to occur. The population and economic activity increases are then allocated to available land in such a manner as to account for the land development policies in existence or desired. Based on the density and location of the various urban activities, zonal aggregate trip origins and destinations are developed. The origins are distributed among the predicted destinations by use of the previously calibrated travel distribution model. The distributed trips



are then assigned to the respective transportation networks. This process places trips between zones on links of the route connecting the zones, thereby yielding traffic volumes. Where the currently available facilities are inadequate to handle the future volumes, new design must be undertaken to accommodate the target year traffic. A program is then developed to facilitate implementation of necessary improvements according to deficiency, continuity and developmental considerations.



CHAPTER II. DEVELOPMENT OF THE THESIS

In examining urban travel, it is appropriate to analyze the individual trips which make up the overall travel patterns. Several dimensional parameters of each trip, alone or in combination, describe the overall effect or importance of the trip to the transportation system. Consideration of importance requires definition of those factors which best represent demand on the system. Since the function of the transportation system is to move travelers (and goods), it follows that any trip that is in the system represents a demand. It further follows that the greater demands are due to movements that are in the system longer. The immediate deduction is that those trips which go the farthest distance in the system are the most important. This may seem to contradict notions of the importance of travel time; but importance to the system is the object of the current examination. Since the transportation system is basically a spatial rather than a temporal entity, length is a better demand indicator than time.

Trip length alone may not be a sufficient indication of importance. A few long trips would constitute fewer tripmiles than a large number of shorter trips. Since importance implies utilization, it is necessary to consider the number of trip occurrences as well as trip length. The most important components of travel to the transportation system are



those with the greatest lengths and occurring most often, for they provide the greatest trip-mile utilization.

The third dimension in travel patterns is concerned with time of occurrence. The time when a trip is undertaken, relative to the timing of other demands on the system, is a significant factor in trip importance. Since the system is dynamically loaded, i.e., trips continually move, stress occurs when the arrival rate on a system component exceeds the ability of that segment to adequately process the trips. The time rate of occurrence, or volume, relative to the processing capability, or capacity, indicates the importance that travel of a specific class assumes in patterns of travel on a transportation system. If the occurrence rates vary significantly throughout the day, the system will be less than ideally efficient, unless the distributions are complementary rather than cumulative.

A fourth dimension of travel is utilization of facilities. The importance of a particular trip is a function of which portion of the total system it uses. Trips made on system segments commonly traveled by many other trips contribute to high stress loadings. The same trip, made on less heavily traveled facilities, would contribute less to system stress and to more efficient system use. Whether or not a trip occurs on the major street system contributes to the importance it assumes in regard to the entire transportation system.



The foregoing importance dimensions are descriptive parameters common to every trip. Their conditions determine the level of significance each particular trip assumes in total urban travel. Certain other descriptive factors are merely elements of trip character rather than importance. They are used as a means of classification to facilitate analytical examination. These factors have traditionally been treated as dimensions, despite their true character as descriptors. The principal descriptive factor is trip purpose, the reason travel is undertaken. Each of the many possible reasons for travel can usually be classified in one of five general groups:

- 1. Work
- 2. Shopping
- 3. School
- 4. Personal Business
- Social-Recreational

Classification by purpose implies that a difference in the motivation of travel affects the character of trips that result. The importance parameters assume typical conditions for each of these purpose groups. The fact of this effect is apparent from examination of travel survey data in any urban area. For this reason, trip purpose is usually a means of stratifying trip groups in order to better examine, evaluate, and predict their parameters.

Mode or means of travel is also used for analytical classification. The most basic modal breakdown is between



highway and transit; under each of these may be subgroups. Highway travel may be broken into truck, taxi, and automobile trips. The latter may be further split into driver and passenger groups. The breakdown of transit travel depends on the diversity of the transit system. The mode groups define different, and usually exclusive, parts of the entire transportation system. The mode chosen for study reflects the orientation of the particular transportation analysis undertaken.

In examining which components of total urban travel are most important to transportation, it is first necessary to define the system of interest. The present research is concerned with private vehicles and non-transit passengers on the highway system. The next step involves comparing the parameters which measure the effect that trip groups have on the transportation system. It is well known that in most urban areas the single most frequent trip is to the place of employment. The journey to work characteristically exhibits the greatest average trip length (Smith, 1961). Work trips also occur in a much more clustered manner than other travel. It follows that the single most important trip group for the highway-person transportation system is that concerned with the journey to work.

Examination of the total importance of transportation should give proper consideration to effects other than those on the transport system itself. Transportation has extensive ramifications for the larger aspect of community activity.



The community as a social entity is totally dependent on economic activity for its existence. The factors most representative of economic activity are goods movements, communications, and person movements represented by the work trip. Except in a few very extraordinary cases, the number of persons engaged in productive or service activity at a given location presents an excellent demonstration of the level of economic involvement. Work travel then is important to the community as a whole because it represents the lifeblood circulation.

The journey to work is also important to the individual elements of urban existence: the individual persons and their households. It represents the means of livelihood of the household and takes precedence over other travel. The place of residence depends to a large extent on location of the work place. The work trip usually occurs at least twice daily, on weekdays, for each wage earner. At least one person in every household is engaged in some form of income producing activity. Work travel occurs at the same time every day. It usually uses the same routes since the travel orientation in most cases doe not vary. Work travel determines the need for purchase of an automobile. Trips other than work are fitted around the availability of the family car and the time of travel to and involvement in work.

The journey to work is then as important to the individual and the community as to the transportation system. It is obvious that this important portion of the total travel



pattern should be pre-eminent in the planning for total transportation. Because of the level of significance, it could even be rationalized that system planning should center on work travel, leaving complementary system usage to the travel which supplements personal and community activity.

The importance of the journey to work had been recognized by many persons at least as early as the beginning of the current century. The concern first occurred in older, more established countries where development had reached such a density that travel problems were occurring. Several European countries and Japan examined work travel patterns, primarily in connection with census studies. Britain also realized the significance of work travel, as evidenced by a 1921 census study. When it came time to plan for rebuilding the destruction effected in World War II, several more British studies were undertaken. Parkes (1941) examined the location of homes and workplaces in relation to the journey to work as a part of postwar planning for Birmingham. Liepmann (1944) considered certain economic aspects of the work trip relative to the mobility of workers and their choice among alternate places of employment. Further work was done by the London Transit System (1949) and by Glass (1956) and Westergaard (1957).

The problems due to work travel in the United States were not severe enough to cause concern until World War II.

The allocations implicit in rationing and the traffic volumes to defense plants motivated investigations of war worker



travel. The problems due to prosperity following the war generated more interest in the patterns of work travel, but concern was not yet sufficient to motivate government action. Using information from traffic origin-destination (O-D) studies, several people began to examine the problem. Carroll (1950) suggested that "forces" tending to minimize the length of the journey to work have a concentrative effect on urban residential arrangement. Work by Adams and Mackesey (1955) and by Beyer (1951) considered the commutation to work in small urban and rural areas. The continuing prosperity and concomitant increase in travel motivated many studies of the patterns of all urban travel, each of which placed increasing emphasis on work travel. The obvious significance of work travel in the pattern of all travel was behind inclusion of questions regarding the journey to work in the 1960 census. Further concern is evinced by the consideration of amplifying the previous queries for use in the 1970 census (Bureau of Public Roads, 1966b).

Recent works by Loewenstein (1965) and Lapin (1964) have indicated further recognition of the important role of work in total urban travel. Loewenstein presents an analysis of locational aspects of urban residences and workplaces. His interest in location implies a recognition of the properties of the work trip. He implies that his concept of distribution may present a feasible approach for eliminating conventional travel survey procedures (Loewenstein, 1965). He has retained nearly the same concept of trip distribution accepted by



transportation planners, but his ideas of the requirements and use of travel pattern information for expressway or transit planning appear quite deficient. This is emphasized by his allusion to "trip distribution" procedures when he actually discussed O-D studies. The approach he proposes would estimate the interzonal distribution of work trips as a percent of total trips originating in a zone, based on proportions derived from O-D data of several cities. The procedure would designate as few as five zones (or rings) in even large cities and utilize a single set of distribution factors for all cities. In comparison with patterns derived from an O-D study the approach badly overestimated intrazone trips in four of the five zones, and in only one case approached an acceptable tolerance in estimating total zonal destinations (Loewenstein, 1965). Loewenstein (1965) admits his procedure is crude and merely an attempt to develop a new approach to travel survey. As such, it is consistent with the philosophy of the study reported here.

Lapin's study emphasized the work journey itself, but included analysis of the place of work and residence. He investigated the relations between the several characteristics of work trips and their termini. He also defined the character of several parameters of work travel and related these to changes over time. His objective was apparently to increase the knowledge of this important fraction of total urban travel. Although no direct allusions were made, interpretation of his presentation in the context of the current



research detected inferences that the journey to work might indeed present sufficient basis for total travel representation. Lapin's work had excellent basis and was very thorough. His implications regarding work trip character and prediction could well be the basis for application of the technique proposed in this report. He also suggested that the vast compilation of data on the work trip and all travel could well lead to future synthetic predictions.

The importance of the work component in the total daily pattern of urban travel has been recognized by many others. The Bureau of Public Roads underscored the place that the work trip holds in this respect. "Major emphasis should be placed on that segment of travel which has the most influence on the design of transport systems." The "work trip is the most ritualistic," the "most free from deviations," and "work trips logically end at jobsites" (Bureau of Public Roads, 1966a). Work travel is the most stable component of a community's daily travel (Smith, 1961). The consistency in work travel character and patterns between cities as pointed out by Lapin has also been emphasized by Voorhees (1958). It is this common knowledge which permits omission of countless articles which re-emphasize these facts. Bouchard and Pyers (1965) found less discrepancy in travel patterns for work than for any other purpose.

Lapin (1964) has suggested that the emphasis on travel analysis should be placed more on the destination terminus than has historically been the case. It is at that point



where the most difficult traffic problems occur. Stewart (1948) has suggested the existence of a worker potential field about each workplace. This situation was corroborated by Lapin's destination analysis, and was therein likened to the assumed gravitation field about the residence (Lapin, op. cit.). The residential distribution of central business district (CBD) workers about the CBD approaches the distribution of all residences about the CED. For non-central areas, however, worker residences are more clustered about the work place (Carroll, 1952). This situation is attributed primarily to the better established transportation oriented to the CBD (Mitchel & Rapkin, 1954). The newer peripheral centers do not have the historic transportation base, so workers must cluster nearer to them in order to realize the same level of accessibility (Mitchel & Rapkin, ibid.). The distributions of residences about workplaces are similar in most urban areas (Loewenstein, 1965). But account may have to be taken for variations within specific urban areas by stratifying according to work type and social group (Loewenstein, ibid.: and Voorhees, 1955). The influence of the social class is seen in ethnic clusters in larger cities. Lapin discusses Osofsky's (1959) approach to a distribution function, and then proposes a similar solution based on a hyperbolic rather than a linear form (Lapin, 1964). This approach is apparently his recommended basis for prediction, although he occasionally alludes to the gravity relationship.



The distributional patterns of work travel are dependent upon the income of the traveler, the mode of travel, the peaking of trips, and the distribution of employment opportunities about the work place (Voorhees, et al., 1966). Lapin (op. cit.) proposed that these characteristics are sufficiently consistent to be amenable to easy prediction. This is one of the most attractive aspects of a work trip oriented approach to travel analysis. Work travel is consistently proportional to total area population. The consistency of this relation between work travel and population is affected little by income or car ownership (Smith, 1961). People must have a livelihood regardless of the rate of earnings or the means of getting there. The number of work trips increases slightly with decrease in occupation class (Shuldiner & Oi, 1962). This is due to "moonlighting" necessary to support a household on a low income skill. Work trips per household increase with household size but at a slower rate than total trips (Shuldiner & Oi, ibid.). The increase in travel potential with family size is realized primarily from the non-work trips.

The labor force constitutes around 40 percent of the population in most cities. In a study of 50 cities, work and business trips accounted for nearly 40 percent of the travel to and from the home (Curran & Stegmaier, 1958). The average family makes seven trips daily, about one-third of which are for the work purpose (Voorhees, 1955). A study of day by day variations indicated that the number of internal person work



trips per day varies least of all purposes, only - 4 percent from the mean (Sullivan, 1963). Contributing to the work trip problem is the low vehicle occupancy rate that occurs for work trips. It is about three-fourths of the average for all purposes and less than that for any other single purpose (Lapin, 1964).

The lengths of work trips in an urban area depend on the size and physical structure of the city, the character of the transportation network and the socio-economic situations that prevail (Voorhees, et al., 1966). In Toronto longer trips were made more often for work than for other purposes (Hill & Dodd, 1966). The different distributions of worker residences about workplaces in the CBD and the peripheral areas lead to variations in work trip lengths (Lapin, 1964). Hoover and Vernon (1962), however, claim that family amenities are rated more important than work access when choosing a residence. The variation in work trip length has been shown to be related to the 0.6 power of the distribution of opportunities (Voorhees, et al., 1966). The value of the exponent or friction factor is significantly lower for work trips than for any other trip purpose (Whitmore, 1965). This is a further demonstration of workers' willingness to travel farther in order to obtain the right jobs, and yet maintain a desirable atmosphere for their families.

The importance of work travel is emphasized by its distribution over time. Perhaps distribution in the common sense is somewhat of a misnomer in this instance, for the work



travel in urban areas is characterized by its clustered occurrence. These clusters of trips are commonly termed "rush" hours. In the terms of traffic engineering they are known as "peak" hours, for during these periods the maximum consistent hourly volumes occur on the transportation system. It is peak hour volume or an approximation to it that is used to design transportation facilities. The peak hour on a given transportation segment may not always be due to work travel. But on an extremely large number of transportation segments within most urban areas, the peak hour volume is composed primarily of work trips (Spiegelman & Duke, 1963). The U. S. Bureau of Public Roads (1966a) has stated that "work trips alone constitute the majority of trips" during the peak hours.

9:00 in the morning and from 4:00-6:00 in the evening. They are composed principally of work trips, but shopping travel is also present in the afternoon. Auto driver trips are greater in number in the afternoon peak, but trips by all modes are greater in the morning peak (Wynn, 1959). This situation can be attributed to the high volume of school trips by bus included in the morning peak hour person travel. In Baltimore 60-70 percent of the p.m. peak travel was for work, while 20 percent was for shopping (Knox, 1962). During these four hours of heaviest travel, 39 percent of all auto trips and 49 percent of all transit trips occurred (Wynn, 1959). School trips account for 60 percent of the non-auto



driver peak, 8:00-9:00 a.m. About 55 percent of all employed persons travel between 8:00-9:00 a.m., 80 percent travel between 7:00-10:00 a.m. (Williams & Robertson, 1965). Variations in the proportion that peak hour is of total daily travel can be attributed to land use at points of origin and destination (Voorhees, 1958). The morning peak volume is fairly consistent for all week days; the evening peak volume is slightly (10 percent) larger on Friday than during the rest of the week (Nixon, 1961). Variations in the proportion that peak hour is of total daily travel were attributed to the effects of capacity and total day volume (Nixon, 1bid.). The proportion that the peak is of the total day is affected by city size (Lapin, 1964).

A peaking tendency of the magnitude observed for work trips does not occur for other major purposes. One explanation may be complementary use of the system. Non-work trips (except school trips) have even been observed to be uniformly distributed throughout the day (Smith, 1961). In fact, travel purposes other than work and school only become significant after 9:00 a.m. (Williams & Robertson, 1965). School trips, although significant in person trip considerations in the morning peak, are not really worthy of extensive concern. They were primarily short, local trips on other than major streets and do not contribute to major congestion of the system. The composition of the evening peak period has historically included substantial commercial or shopping travel (Wynn, 1959). This has been primarily due to suburban



area shopping that occurs during the journey from work to home (Voorhees, 1958). Recent trends to evening shopping hours in large suburban centers have led a shift of the evening shopping peak to 7:00-8:00 p.m. rather than 4:00-5:00 p.m. (Wynn, 1959). In Toronto this broadened the effect of the evening peak hour (Hill & Dodd, 1966). The peak of travel to shopping in the CBD occurs at 10:00 a.m. leading to a maximum accumulation of persons in the CBD at between 12:00 noon to 2:00 p.m. (Voorhees, 1958; Lapin, 1964).

The peaking of work trips may require more than the current aggregative analysis. There appears to be sufficient variability in travel character between the several employment groups to warrant examining them separately. Lapin has given considerable time to this aspect of travel character (Lapin, op. cit). The work travel of top social classes is more concentrated in the peak hour due to similarity in job hour and vehicle availability (Williams & Robertson, 1965).

The characteristics emphasized in the preceding discussion are to be the subject of the research reported henceforth. It should be apparent that the importance, regularity and stability of work and peak hour travel are such as to be amenable to their use as a means of prediction. These factors lend themselves well to forecasting. The importance of these elements (work and peak hour travel) are underscored by the extent to which they have been examined by others. But there is one thread missing. In spite of all these obvious advantages no emphasis has been placed on the singular



use of work as a means of prediction of total urban travel. It is not the purpose of this study to imply that work patterns are identical to the patterns for all travel. It is merely proposed that work travel is so demanding of transportation that its satisfaction will define a system sufficient to serve other purposes or will adequately represent a system that does in fact serve all travel. Similarly, the use of the peak hour as a design standard has been relegated to obscurity for qualitative reasons. It is proposed that the work to follow will examine the feasibility of the approach to transportation planning based only on work and peak hour travel.



CHAPTER III. PROPOSAL OF THE HYPOTHESES

It is important to discuss the concept of complementarity as applied here to urban area travel. Complementarity is based on the principle of impenetrability, which states that no two material objects can occupy the same place at the same time. It implies that, of two objects competing for positions, that which has the greater utility for the position will take priority in assignment of the location sought. Ullman (1956) originally proposed the concept in connection with activity occurrence. As applied to urban area travel, it implies that purposes which have rigid restrictions of some type (e.g., work) will take precedence over those with less demanding constraints (e.g., shop). Given a fixed transportation system of defined space capacity per unit time, trips with the most inelastic requirements will attain either total satisfaction of their demand or total usurpation of system capacity prior to relinquishing system usage to other trips.

The ramifications of complementarity may be observed in the purpose-time distribution of urban travel. Work trips occur during certain time periods because of the importance of having a job, the punctuality requirements of employers, and the various factors which promote common business hours.

Work trips dominate these periods because of their large volume in relation to system capacity and the resulting reluctance of persons to travel for less necessary purposes during the same hours. Trips of non-work purposes occur during hours when they do not have to compete with work trips for the transportation system, since their timing is less critical.

Based on the complementarity between purposes over time. there is a concomitant relationship between purposes over space or street system capacity. Peak hour traffic, by definition, stresses the highway network more than traffic during any other hour in the day. The travel pattern during the peak hour may thus be assumed to provide the best definition of the highway system, since the maximum stress situation provides the optimal condition for defining any system. The principle of least resistance implies that total travel in other than the peak hour will use the same basic system, to the extent that origin and destination permit. It follows that since the work purpose dominates peak hour travel, it is responsible for defining the basic street system, the same system that is used by other purpose trips in other than the peak hour. This concept of the representative nature of work travel patterns is the basic thesis of the research reported here.

The existence of a single basic system is quite apparent upon examination of travel patterns in urban areas of most any



size. Peak hour traffic jams occur on its constituents. Shopping centers develop along its margins. Industry insists on its close proximity for accessibility and advertising advantages. Hosts of commercial establishments develop along it. Residential areas, although not likely to locate on it, want the accessibility to other land uses that it provides. The fact that all traffic uses the one system is apparently due to the universal appeal of the system to every type of non-residential establishment.

The correspondence between the work trip system and the non-work trip system cannot be assumed perfect. The non-work group contains more purposes and, therefore, has more possible destinations. Non-work trip length is also less, in general, than that for work. This implies relatively more use of peripheral (local or collector) systems by non-work trips than by work trips. Non-work trips, therefore, contribute a greater proportion of the volume on the peripheral system than do work trips. But these systems also contain substantially more miles of street than the major street systems and their design is based on land access rather than traffic service. The resulting number of vehicle-miles of travel is extremely low, implying the relative unimportance of the peripheral systems.

The complementary use of a single basis street system by the several purpose groups implies a highly desirable degree of efficiency. Such efficiency is certainly not complete, nor is it conceivably a conscious objective of urban



travelers. It is more probably correctly described as a "blind" optimization procedure by which trip makers unwittingly achieve a balance between their multiple resources of time, energy, comfort and convenience. The above emphasizes the importance of the major street system in the transportation planning process.

The representivity exhibited by the work trip in regard to urban travel patterns can be expected to increase as city size decreases. Such an effect would be attributable to the lower absolute volumes of non-work travel and to the less highly developed nature of the peripheral streets. The latter situation requires non-work trips to use the work or basic street system for a greater portion of their length, thereby increasing the amount of agreement observed in street importance. The purpose of trips on streets near a zone centroid is significantly influenced by the function of the zone.

Work travel is a very convenient variable for analysis. The work purpose trip is the most stable and ritualistic component of urban travel (Bureau of Public Roads, 1966a). It occurs between the two most readily predictable land uses. It serves only two activities, the residence and the workplace, and is less influenced by separation than most any other purpose (Bouchard and Pyers, 1965). Work trip generation rates are characteristically stable and predictable. Work is the highest density activity in the urban area. In general, no other function concentrates more people per unit



of area than the work process. Because of this property, there is a related concentration of traffic at the workplace. Traffic intensity on facilities serving centralized work places or very large single workplaces can extend for considerable distances.

The important role that work oriented travel assumes in the total picture of urban transportation should be apparent from the previous discussion. Travel in nearly every city is dominated by the journey to work. Only in cities whose function is somehow extraordinary can instances be seen where work travel is of lesser importance. The characteristics of work travel as a sufficient representative of the total urban travel pattern are essential elements of predictability. Based on the attributes enumerated above, it is proposed that work travel would be a valid and sufficient means for prediction of total urban travel patterns. It is hypothesized that, for use in the urban transportation planning process, the pattern of travel developed using work oriented trips alone presents a sufficient representation of the major street system used by all urban travel. It is further proposed that this implication is nearly as valid for home-based work trips as for all work trips. These assumtions are to be tested by the several statistical procedures described hereinafter.

The feasibility of representing the distribution pattern of trips of all purposes by the distribution pattern of work purpose trips alone will be tested. These travel



patterns will be examined over the entire system of major streets and highways. Zone-to-zone travel patterns cannot be used as the basis for testing, however, since they are strictly dominated by the functions of respective zones. Since the desired results of the prediction procedure concern the major street system, the links of the network representing this system will be used as the basis for comparison. Travel of the respective purpose groups will be assigned to the network, and a test of the representivity will be made on a link-by-link basis. In this manner, the zone centroid influence is virtually eliminated in favor of testing the conditions on the major street network.

Throughout the previous discussion it has been implied that the peak hour is an important factor in urban transportation. Similarly, it was argued that work travel is a dominant aspect of the peak hour. It is then apparent that system design based on peak hour volumes may be feasible. The principal advantage of this approach is that the result is a functional basis for design. The methods by which average daily traffic (ADT) volumes are obtained in current planning practice contain considerable inherent error. Further compounding this by introducing a factor to obtain a design hour volume seems unreasonable. By the time factoring has been accomplished, the care of a very expensive survey might well have been wasted.

The peak hour is the one most consistent and significant point of stress on the transportation system. It would



seem only too obvious to deal directly with the maximum loading condition rather than to factor to it. The most significant argument raised against peak hour oriented design is that peaks occur at different times at different locations and for different purposes. Such a situation can only be examined empirically, but it seems that the theory of complementarity would be as applicable in this situation as previously. To follow this argument, assume a system based on a single peak hour, say for work. Beyond this system, heavy non-work volumes, whenever their peak, could be accounted for by specialized surveys or analyses. The residential end is served by the work oriented system. Regarding the time shifts of peak volumes, as long as the maximum volume has been defined relative to the system, the location of its occurrence is immaterial. The trips involved are still full trips, from origin to destination, and will sometime during the peak hour contribute to the volume. This situation, of course, is contingent upon a peaking definition such as used here, i.e., maximum trips on the entire system (see Chapter IV).

The hypothesis proposed for test in this second instance involves the representivity of the ADT loadings by peak hour loadings. Continuing them, the reproducibility of peak hour loadings using a survey of work trips alone might also be feasible.

The hypotheses proposed are directed at elimination of .

the home-interview survey technique and replacement thereof

by a special survey of work purpose trips only. Such a survey



could take any of several forms. One of these, useful principally for an updating function, involves the current proposal to obtain workplace information of employed persons as a portion of the decennial census (Bureau of Public Roads, 1966b). The single question currently proposed would ascertain the address of the location at which the interviewee was most often employed during the week preceding the interview. The possibilities of such an application may be further examined after evaluation of research such as that proposed here. Study of the feasibility of this situation has also been undertaken by the Bureaus of Public Roads and the Census.

One of the main reasons interviewing has traditionally been conducted at the dwelling unit is the requirement to obtain trips of all purposes by all members of the household. Quite naturally if the interest in trip making were confined to a single purpose of travel, the interview place should if possible be oriented to the destination of trips for that purpose. Thus evolves the concept of the destination place interview.

To temporarily diverge somewhat, the concept of a destination place interview can be seen on reflection to be of quite obvious utility. The destination place of any trip purpose group is in every case at least as densely attractive as the residential end of the trip. In most situations the very concept of activity or service provision on a production line basis requires high density trip confluence. The question



immediately arises of why interview at the household? The ramifications are not immediately obvious although certainly valid. They involve such factors as trip generation potential of the household and its excellent predictability as regards future time travel estimation. Also to be considered is the opportunity to obtain information on travel by all persons in the household. The home is the terminus for 70 percent of trips in the urban area. This adds to the body of knowledge of urban area travel, but its true utility may be questioned in light of the theoretical implications of complementarity. Because of the high cost of home-interview surveys, only a small sample is generally taken. As a result the number of reported work trips, the most important purpose group, is small.

On the other hand, the implication of a concentrationor destination-place travel survey is only too apparent. What
traveler has not at some time or another been caught in a
traffic jam in the central business area or near an industrial plant at shift change or near the site of a major recreation area? At all these locations a service or activity is
offered which attracts persons from all residential areas to
one relatively small location. Such activity can exist only
if major interest can be focused upon it and transportation
service provided for it. Very rarely, if ever, do traffic
problems occur at the location of residence; probably the
only case, concerns high density residential areas, which,
of course, fall into a destination place type situation. The



object of transportation planning should be provision of service at locations of maximum stress, i.e., the points of traffic concentration. But for the facets of predictability and multipurpose considerations, the household interview approach is somewhat less attractive than the destination place survey. Destination place surveys could also, depending on the procedure, obtain measures of non-home trips. The problem reverts again to consideration of the theory of complementarity. If the basic system indicated by home-based trips is developed, it will function for non-home-based travel as well, providing the exceptions are taken into account.

The consideration of destination place interviews is not unique or novel. Cherniak (1960) proposed such an approach to travel surveys. He suggested that "for planning future urban expressways, sample trip and correlative data should be assembled, not in the homes but in areas where people are concentrated during the day." "Those are the areas on which traffic converges, where traffic is concentrated...and where...additional...vehicular capacities are... needed (Cherniak, ibid.)." Cherniak further impled that generation studies should be oriented to the concentration points rather than the residential destinations, and that all information from travel surveys should reflect the patterns of peak hour travel. In discussion of the proposals by Cherniak, two points of note were raised. Campbell (1960) suggested that destination point interviews



would yield "no total frame of reference" as a quide or basis for transportation planning. The implication is that total trips or total vehicle miles of travel are not available when surveying only one facet of travel. No demonstration is given, however, of why total travel is so important to system analysis, especially if valid patterns and design hour volumes are available. Lynch (1960) implied that a concentration place interview might involve a less "scientifically selected sample" than the home interview. It is not apparent why "scientific" selection of a sample could not as readily be applied to concentration place interviews as otherwise, as long as the nature of the population is considered. Lynch also decried the lack of a universe for expansion and questioned the ability to evaluate result accuracy. His experience with such work, however, may cause concern in regard to his implication that employer cooperation might be difficult to attain.

The applicability of the destination-place interview concept to work oriented travel points to surveys at the place of employment. Depending on whether or not non-home oriented trips are necessary, such a survey would take one of two forms. A survey of home-based travel would be confined to examination of employee records for coding the address of workers' residences. Such a procedure eliminates all response error due to interviewing. Survey costs are reduced to the cost of coding the information. Should it be desirable to obtain information on non-home trips, an alternate procedure

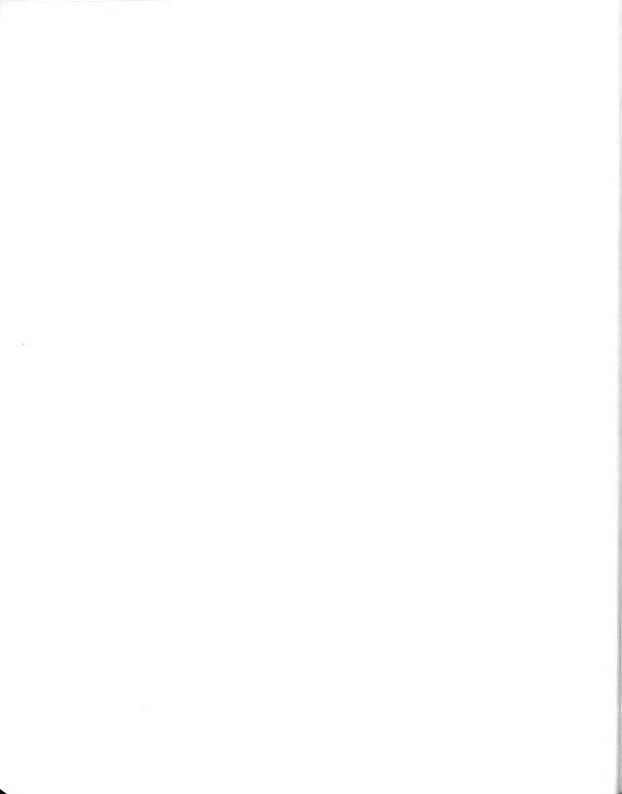


would be to utilize either a questionnaire or an interview procedure. If vehicle trip information was desired, a means of eliminating non-drivers could easily be determined.

Certain advantages accrue from a survey of this type in comparison to the household interview. The interviewer is dealing with the traveler in an environment which implies good recall of the trip. The trip is very ritualistic and stable; most work travelers could define the exact routes they took to work and why these were chosen. Employer permitting, the interview or form completion should be done during working hours. The psychological implications of the company's interest being sufficient to dedicate "its" time and the undeniably desirable opportunity for the employee to stop work lend an atmosphere very conducive to the ultimate in cooperation.

Needless to say such interview procedures require an excellent public relations program directed to the employers in order to obtain the ultimate in their cooperation. Fulfillment of such ideals may be realized in smaller cities more than in larger cities. The cooperation will probably be better from large than from small employers.

The ritualism of the work trip will provide a reduction in the experimental error of the survey. Contact of a greater proportion of those involved in trips reduces sampling error. A one in twenty sample, such as was used in Indianapolis, yields five percent of the trips in the area. Interviewing at work and obtaining information on say 90 percent of total



employees within an urban area yields data on (90 percent of trips that are 40 percent of the total) over 35 percent of the trips in the area! Reductions of the various error sources offset standard errors of the estimating procedure used. It is even conceivable that the employment place interview could obtain a good proportion of the information obtained in the home interview, particularly if good design of survey procedures was employed. Further advantages in speed of data collection and reduced travel costs for this procedure are apparent.

There will probably occur in any city studied certain specific situations which cannot be handled by the proposed procedures alone. In such cases modifications or augmentation of the basic survey will probably be sufficient to yield valid results. Traffic studies of concentrated movements or travel pattern studies at major generators would probably provide sufficient additional information to yield adequate system definition.



CHAPTER IV. DATA PREPARATION PROCEDURES

The data used in this study was obtained from the Indianapolis Regional Transportation and Development Study (IRTADS) through one of its sponsors, the Indiana State Highway Commission. The decision to use the IRTADS data was due to its convenience and availability. It was also current, being from surveys conducted in 1964 as part of a planning process approved by the Bureau of Public Roads (BPR). Such approval was important from the aspect that this research was aimed at recommending changes in BPR procedures. The result of the proposed revisions should necessarily reproduce results obtained by the procedures to be changed. Use of data from a study area the size of Indianapolis should imply the generality of any results to any city of similar character. All surveys were complete and the entire data file used by IRTADS to develop transportation recommendations was available for use in this research.

The specific information selected from the IRTADS data file was the travel data from the home interview survey, as coded and punched on "number 2" cards. Among other items, these cards included the information listed in Table 1 which was utilized at least in part in some phase of the present research. No use was made of either the truck-taxi or the



Table 1. Data from Home-Interview Trip Record (No. 2 Card) Used in Various Stages of the Analysis.

- Sample Number 1. 2. Person Number Trip Number 3. 4. Residence Zone 5. Origin Zone 6. Destination Zone 7. From Purpose 8. To Purpose
- 10. Start Time

Passenger Purpose

- 11. Arrive Time
- 12. Mode

9.



external survey data since the principal objective of this work was to attack the home interview survey. The inventory of the 1964 street network as punched in standard (BELMN) format was also obtained. Transit network information was not used because the proposed work was principally highway oriented. Transit trips were included in one phase of the analysis only because no network information was required.

The complete file of home-interview travel data was pre-processed to put it in a suitable form for subsequent manipulation. Dwelling unit (No. 1) cards were not used at all. All trips with an origin or destination outside the study area were eliminated because there was lack of information on the external terminus and the difference in motivation and character between those trips and the wholly internal trips. Without a home-interview survey, such trips could still be tabulated at external cordon interview stations. This deletion eliminated 2416 trips from the basic file. The remaining trip cards were grouped according to home orientation. Home-based trips, having at least one terminus at the residence of the trip maker, were placed in one file and sorted on zone of residence. Non-home based trips, having no terminus in the residence zone, were placed in a second file and sorted on zone of origin. These files were the input for the travel data processing programs for the total day condition.

In order to select a representative peak period the combined home- and non-home-based files with externals deleted



were processed by the program PEAKS (Appendix C). PEAKS scanned the standard trip survey cards for trip purpose, mode, and times of start and arrival. The times were recorded separately according to the mode and purpose of the trip. PEAKS then computed the number of trips entering and leaving the transportation system, by mode and purpose and in tenth of an hour increments throughout the day. PEAKS then aggregated the incremental periods into successive one hour blocks and produced the number of trips in progress, by mode and purpose, for contiquous one hour periods, successively by tenth of an hour increments through the day. The hours during which trips in progress by each mode and purpose reached their maximum were then available. PEAKS also aggregated modes and purposes into total tables. Optionally PEAKS will yield tenth of an hour accumulations only, in order to permit the user to aggregate peak periods other than a single hour.

The output from PEAKS as applied to the IRTADS data is shown in Appendix B. Note that the values shown are card counts, not factored traffic volumes. The peak hour selected for use in the current analysis was that defined for all auto driver trips. This was the condition considered to place the most stress on the highway system. A single hour was selected because the objective in the peak hour phase of this study was to obtain a single hour volume on which to base design recommendations. The peak hour selected for subsequent study was from 16.4 to 17.4 hours (4:24 to 5:24 p.m.).



Processing of the travel and network data involved extensive use and total reliance on the computer program package disseminated by the Bureau of Public Roads for use in operational transportation planning studies. Use of the system was decided upon because of its ready availability and comparatively error free function. It is designed for high speed, high volume processing of the type required by the subject research. The IRTADS data had been coded according to formats required by the system since IRTADS also used these programs. The package is known as BELMN, and is based on the Bell Telphone Laboratory monitor system for the IBM 7090 computer. BELMN consists of a set of programs that feed between one another in a sequential processing setup under user control with a minimum of external supervision (Bureau of Public Roads, 1964 & 1965). The system worked quite satisfactorily in most instances with the principal problems being due to inadequate documentation.

Several decisions regarding format of the final data to be analyzed had to be made prior to the initiation of bulk processing with the FELMN system. It was decided that trips of three specific purpose groups would be obtained in addition to the all-purpose group. The selected purposes were home-based work, home-based shop, and non-home-based work. The differentiation between home and non-home-based trips has previously been made. A home-based work or shop trip is one that has a purpose on the non-home end of work or shop respectively. Home-based trips must be either to or from the



zone of residence; the home orientation at either end determines the classification. Non-home-based trips, however, could reasonably be classified by either the to or from purpose since neither is at the place of residence. For the current study a non-home-based work trip is one having a work purpose at either end of the trip.

The three specific purpose categories also included small groups of trips which were of similar respective character. These were serve-passenger trips with passenger purpose the same as the specific group in which it was included. addition was due to the action by IRTADS interviewers of "linking" trips at the time of interview. Thus the normally defined serve passenger trips had been linked out of the IRTADS survey data (Bureau of Public Roads, 1964, pp. IV-4). The nominal serve-passenger trips were "busing" trips and should be included in the respective categories. Servepassenger trips that were not of a busing nature retained sufficient influence from the passenger's purpose to permit assumption of the passenger's purpose at the serve-passenger end. (Linking is the term used to describe the process of removing from consideration an intermediate trip terminus, the purpose of which is subordinate to the final destination purpose.)

The modes selected for analysis were auto driver and highway person, reflecting the decision to maintain a highway orientation. The former represented vehicle trips and could be indicative of traffic volume. Highway person trips



included auto driver trips as well as passengers in automobiles, trucks, and taxis. Use of a person trip orientation in revised survey procedures would, of course, permit development of modal split relations. No transit or school bus trips were included because of the lack of knowledge of a network for either group. Two periods, total day and the single afternoon peak hour, defined the time conditions to be considered.

Further processing of the basic data in preparation for the analysis used the BELMN program package. Program numbers are those of the standard writeups. The shorter of two phases using BELMN involved processing of the street network. The 1964 street inventory had been coded and punched on cards by IRTADS. These cards were then processed according to the diagram in Figure 2. Program PR-6 processed the cards containing among other information the length of, speed on, and node number at the terminals of every link in the street system. The description of the network prepared was stored in binary format preparatory to further processing. Program PR-1 scanned the network description for link direction and travel time and the terminal node numbers. It then proceeded to build "trees" from each zone to all other zones in the study area. These trees are the link-by-link description of the path taken in moving from one zone to another. Since there could be a very large number of such paths if there were no restrictions on their selection, the path which exhibits a selected type of optimality is defined. For the



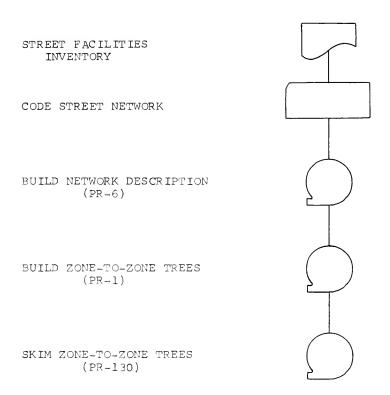


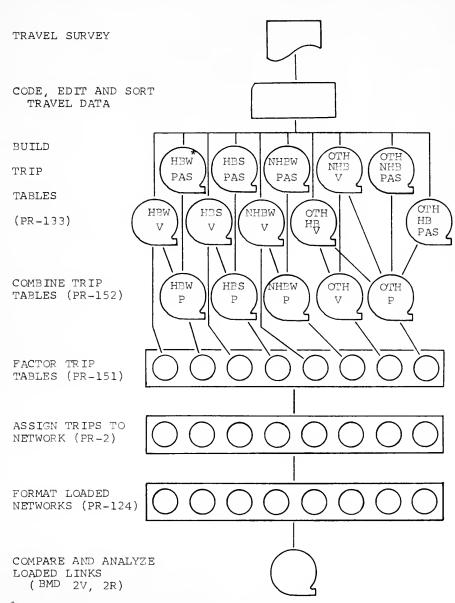
Figure 2. Developing the Street Network Data.



current study, the minimum travel-time path was chosen. Travel-time includes terminal time at both ends of the trip. The procedure for selection of this optimum path is based on an algorithm developed by Moore (1957). PR-1 prepared a binary tape describing the minimum time path trees, link-by-link, for all zones in the study area. Program PR-130 further processed the binary tree information by summing the time to traverse the links in each tree, producing the accumulated time to move between each zone pair on the minimum time path. This zone-to-zone cumulative time is known as a "skimmed tree." The skimmed tree data were used as described in Chapter V. The network description and zone-to-zone tree data were used as described below.

The major utilization of the EELMN programs was concerned with processing the travel data. Figure 3 shows the procedure employed and described below. The necessary preprocessing of the trip cards was described previously. Input to program PP-133 was in two phases, home-based and non-home-based trip cards. This separation was necessary because of the requirements of PR-133. The procedures were, however, essentially identical, and the results were combined at a later stage. PR-133 accumulated the trips from each origin to each destination zone according to groups based on the trip purpose and travel mode specified previously. Output from PR-133 were "trip tables," cumulative zone-to-zone movements by purpose and mode. The normal PR-133 expansion of the trip data was not used in order to retain exactly the





^{*} See page 105 for abbreviation key.

Figure 3. Developing Travel Data.

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character of the sample. The blanket expansion used is explained subsequently. Because of the nature of PR-133, all final trips tables are complementary; i.e., they do not overlap. Program PR-152 was used to merge certain tables in an additive manner in order to obtain the purpose combinations specified for analysis.

Since trips on links of the network had been selected as the decision variable, the trip tables had to be assigned to the street system. The traffic assignment program, PR-2, uses a factoring procedure to conserve space in core storage and on tape. It divides the actual trip volume by four in order to store the number in a field width of two less positions (binary digits). Such a procedure is not critical for high volumes, but proves extremely damaging in cases where there are many low volumes. Since the trip data had not been expanded as would normally have been the case, the discrepancies here were quite serious. As a means of overcoming the problem, it was proposed to multiply the volumes by twenty prior to assignment. This would permit retention of the exact character of the sample and would eliminate the loss of critical volumes. It would also yield figures of a magnitude similar to observed traffic volumes. Program PR-151 was used to expand the recombined results from PR-152 by a factor of twenty. The entire trip processing and assignment procedure was executed for both the total day and the peak hour situations.



The traffic assignment process, utilizing program PR-2, assigned to each link of the minimum time path tree the zoneto-zone movements given in the trip table. This process, known as "loading the network," accumulated the trip volumes on each link for all zone-to-zone movements. No attempt was made to apply capacity restraint (iterative assignments to account for the change in travel speed with volume) to the loaded networks, since the differences in absolute volumes would have yielded inconsistent results from such a procedure. The objective of the research was to match the control loading condition; that having been accomplished, restraint procedures would be applied to the synthesized loadings. There were sixteen separate network loadings, made up by the twelve specified purpose situations and four totals. The loaded networks were summarized by program PR-124 to obtain a more readily processable output format and a listing of the loading on each link. The output from PR-124 was processed by a data reduction program which summarized for each link the loading under each of the sixteen specified conditions.

Since the principal influence of the hypothesis was intended to be over the major street system, it was necessary to select the links in this group. In order to provide an objective basis for the selection, the functional street classifications developed by IRTADS were used to group the links. The IRTADS system was composed of five groups: local, collector, arterial, expressway, and freeway. Pecause of the small number in their groups, expressway and freeway links



were combined under the latter title. The resulting arterial and freeway groups were considered the major street system. Because of an interest in examining results beyond those on the major street system, the collector group was retained for compatible analysis. All local links, centroid and external node connectors, were removed because of domination of travel on them by the zone represented by the centroid. Links connecting between different groups were considered collectors. The groups at this point represented a modification of the IRTADS system and were referred to as Mod-1 system:

Freeway 231 links, Arterials 307 links, Collectors 2123 links.

Examination of the volume distributions in the respective groups indicated certain conditions for which modifications were appropriate. The link volumes for the auto driver, total day, all purpose condition were chosen as the criterion since they were the best available representation of actual traffic volumes. Based on the estimated standard error of the group, all links with volumes less than 140 were deleted. This was because the true volumes on these links in the average situation might be reasonably considered not different from zero. Links that were the only connection between the system and local links were deleted for the same reason as the local links. Links previously classified arterial or freeway, but having volumes less than 1000, were merged with collectors. Collectors with volumes greater than 5000 but



less than 12,000 were merged with arterials; those with volumes over 12,000 were merged with freeways.

The rationale for these modifications was based on the fact that no capacity restraint was used in the assignment process. As a result, trips were assigned to the absolute minimum time path without consideration for the capacity of the links used. Such a situation would explain the failure of links to carry volumes commensurate with their functional classification. In order to correct for the situation, links having arterial level volumes were defined as arterials, etc. This was the reasoning behind volume considerations when reorganizing the groups. The volume criteria for each group were established by a generalized capacity analysis of the respective street classes.

The Mod-2 system, used in the final analyses, was composed of the following:

Freeways 218 links, Arterials 529 links, Collectors 1793 links.



CHAPTER V. ANALYTICAL PROCEDURES

The analytical procedures employed to test the proposed hypotheses fell into two distinct phases. The first was directed toward establishing a basis for consideration of the second. The second phase was directly concerned with testing the principal hypotheses regarding work and peak hour travel.

Phase one undertook examination of the hypothesis that the several factors of trip purpose, mode of travel, and time of trip do significantly influence the character of person movement in an urban area. The objective was to obtain a quantitative definition, in probabilistic terms, of that interrelationship which most practitioners have come to accept in a qualitative sense. The variables chosen for examination were travel volume and length of trip. These were considered sufficiently representative of the distributional characteristics of urban travel to permit valid generalization and extension of the effects observed.

Travel volume was defined as the number of trips made, where each survey card represents a trip. Trip length was the time required to complete a given trip on the minimum time path from the zone of origin to the destination zone. The purpose of a trip was that indicated on the survey card at the point of destination. Purpose was considered in six groups:



- 1. Work
- 2. Shopping
- Social-Recreational/Eat Meal
- 4. Personal Business/Medical-Dental
- 5. School
- 6. Other

Groups three and four are combinations of purposes originally tabulated separately. The merger was due to assumed similarities in the character of travel for these purposes. Mode of travel was defined in three groups:

- 1. Auto Driver
- 2. Non-transit passenger
- 3. Transit passenger

The definition of transit includes school buses as well as other bus vehicles; there is no other form of transit in Indianapolis. The non-transit passenger group includes passengers in private automobiles, taxis, and trucks. Time was defined in 24 one hour groups. The mean of the start and arrive times reported for the trip maker was used to place the trip in its time group.

The basic data source was the IRTADS home interview survey file (No. 2 cards) consisting of 76,396 records, each describing one trip, wholly within the area, made by a resident of a household selected for interview. The sampling unit for the survey was the household. The households had been selected in a systematic manner from public utility records and represented approximately five percent of the dwelling units in the study area.

The trip records were sorted by zone of trip origin and then by zone of trip destination using the IBM 7094 IBSYS



Generalized Sorting System. The length (in time) of travel from each zone to every other zone was computed from the minimum time path tree by a tree skimming program (PR-130). that is part of the BELMN package. The skimmed tree provides the overall time by summing the times to traverse each link in the minimum time path over all links in the path. It is, therefore, the minimum possible zone-to-zone travel time. The appropriate skimmed tree time was then appended to the individual record of each trip by the program LENGTH (Appendix C), being made an additional permanent part of each trip record.

The nature of the hypothesis to be tested was appropriate for investigation by the analysis of variance technique (ANOVA). This statistical procedure involves classification of the observed variable according to several factors, the object of the investigation being to determine the extent to which the factors affect the observed variable. Thus an observation occurring under a particular set of conditions would be grouped only with observations which occurred under similar circumstances. In the type of study undertaken here, termed a complete factorial, there are the same number of such groups, or cells, as there are combinations of possible conditions (levels) of the factors considered. This procedure isolates the quantitative effect that each factor has on the variable analyzed, but also permits evaluation of effects occurring due to factors acting in combination (interaction).



In order to test the significance of the effects due to factors and interactions, the ANOVA uses an estimate of experimental error, i.e., natural variability, not due to the factors analyzed, to be expected in the occurrence of the variable. One means of obtaining such an estimate in experimentation is to replicate or repeat at least a portion of the experiment, since variability in observations made under identical conditions can be attributed to experimental error. For the present investigation it was decided to select four random subsamples from the basic trip file. These four complete subsamples provided the necessary estimate of experimental error. In order to simplify the sample selection procedure, the observation selected for testing was the mean trip length value over all trips in each cell.

The equations representing the analyses, commonly called analysis of variance models were:

$$X_{ijkl} = \mu + P_i + M_j + T_k + PM_{ij} + PT_{ik} + MT_{jk} + PMT_{ijk} + \epsilon_{(ijk)l}$$

where:

X ijkl represents trip volume or trip length, depending on the analysis, for the i-th purpose, by the j-th mode, in time period k, for the l-th subsample;

 μ is the respective overall mean;

 P_{i} is the effect of the i-th purpose, i=1,...,6;



is the effect of the j-th mode, j=1,...,3; $T_{k} \qquad \text{is the effect of the k-th time period (hour),} \\ k=1,...,24;$ $PM_{ij} \qquad \text{is the effect of the purpose-mode interaction;} \\ MT_{jk} \qquad \text{is the effect of the mode-time interaction;} \\ PT_{ik} \qquad \text{is the effect of the purpose-time interaction;} \\ PMT_{ijk} \qquad \text{is the effect of the purpose-mode-time interaction;} \\ e_{(ijk)1} \qquad \text{is the experimental error;} \\ 1 \qquad \text{is the number of the subsample, l=1,...,4.}$

It will be noted that all effects are fixed, i.e., they are not random samples from an infinite population of such values. The inference permitted can, therefore, only be considered applicable for those levels of the respective factors included in this analysis. The analysis described is known as a complete three-factor factorial analysis with four observations per factor level combination. The assumptions required for use of the ANOVA were considered sufficiently satisfied. The effects were additive because of the simplicity of the counting process. The experimental errors were considered normally and independently distributed due to the averaging of the trip length variable.

A theoretical consideration at this point involved the inference space of the results. The objective was to imply validity not only for the city of Indianapolis, but for the nation as a whole. Such an implication is valid if the trip data used is considered a randomly selected single cluster sample from a nationwide population of trips.



The random subsamples of the basic systematic sample may be considered random samples of trips in Indianapolis. When four subsamples of 10,000 each were drawn from the original 76,396 trips that represented a five percent sample, each subsample was effectively a sample of less than one in one hundred and fifty and was considered drawn from an infinite population. Under these circumstances infinite theory was closely approximated, and finite population correction was not necessary.

Selection and processing of the four samples was accomplished by the program SAMPLR (Appendix C). Input to SAMPLR was four sets of unique, sorted random numbers developed by the program RANDOM (Appendix C), and the sorted trip card file, augmented with trip lengths. The four files of random numbers were stacked on magnetic tape for use by SAMPLR.

SAMPLR read the random numbers and, based on each, selected the data occurring in the designated location of the trip card file. The records selected were tabulated by purpose, mode, and time of trip. Each record used was deleted from the input trip file, and those remaining were written out to await selection of the next sample by the subsequent pass of SAMPLR. The sample selection process was repeated four times. The results of one sample are shown in Appendix B. These data were punched on cards in preparation for their analysis.

The complete factorial analysis of variance computations were executed by program BMD-2V (UCLA, 1966a). The output



from the computation is presented in Tables Al and A2 of Appendix A. The results of the tests for significance are presented and discussed in Chapter VI.

Phase two of the analysis involved testing the principal hypotheses, concerned with the use of work and peak hour trips to represent total daily travel. The objective of the analysis was to determine the degree to which trips of a single purpose or a particular time period could be expected to reproduce the pattern of all travel in an urban area and define the transportation system used thereby. Travel volume on individual links of the highway network was the decision variable selected; the form that the variable took depended on the analysis performed. Reflection on the objective of the research pointed up the necessity of retaining the all-purpose loading as the control condition, against which the hypothesized revisions would be tested. The nature of the situation, with the variable to be predicted containing the variable used to predict, indicated that a regression approach would be most appropriate.

The extent of the regression analysis required was investigated by a modification and extension of the analysis of variance performed in phase one. The objectives of this second ANOVA were to determine which factors should be included in the regression models and what different models were necessary. This analysis was designed to test the effect on individual link volumes of change in the factors purpose, mode, and time.



Definitions of the factors and variable for this analysis were modified from those applied in the first investigation.

In this analysis purpose was considered at three levels: home-based work, non-home-based work, and non-work. This reflected a split of the previous P1 (work) and combination of P_2 to P_5 . Mode was included at only two levels, transit trips having been deleted. The time levels were redefined as peak hour, one particular hour, against non-peak hour, the remaining 23 hours combined. The observed variable was relative assigned traffic volume. This variable was obtained by assigning trips (variable in the first analysis) to the links of the highway network and dividing each resulting link volume by the link-trip total over all links for its particular factor level combination. This manipulation eliminated between cell differences attributable only to differences in absolute total volumes of trips observed for respective purposes. The effect of the absolute totals had been examined in the first analysis; the second analysis was to examine the degree to which selected observed effects extended to the highway system. The resulting variable, termed link-relativeimportance (LRI), was indicative of the status the particular link assumed regarding movement of traffic in the area.

If there was no significant difference found due to purpose, it could be reasoned that each link was as important for moving work trips as for moving other trips. Lack of significance due to mode would imply that passenger travel is distributed on the system in the same manner as vehicle travel.



And no significance attributed to time would infer that peak hour traffic uses the same links as non-peak movement. Should any main effects not be considered significant, regression analysis of that situation would not be necessary. If the main effects were found significant it could be reasoned that sufficient difference occurred between purposes, modes, or time periods for these factors to be considered in the regression analysis.

Of particular interest in this ANOVA was whether the significance of interactions carried through from the first analysis. An interaction implies that the results of varying one factor under the constant level of another factor might not match the results of identical variations of the first factor under different conditions of the second factor. Thus, a significantly different relationship might be found between volume and purpose for auto driver trips than for passenger trips. Interaction significance would imply a need for different regression models at each level combination of the interacting factors. This analysis would yield a rational basis for the form of the regression equations and contribute to the understanding of underlying relationships.

Consideration of two requirements of the analysis of variance was necessary.

The ANOVA procedure bases its tests of significance on properties of the normal distribution and requires that the experimental error within the classification groups or cells be normally and independently distributed. Tests of this



condition utilizing the Komolgorov-Smirnov (K-S) test for goodness of fit (Ostle, 1963), indicated that the raw LRI values were not normally distributed. It was found, however, that after a square root transformation of the raw data was carried out, the K-S test showed significant (5 percent level) departures from normality in only a very few cases for the arterial and freeway classes of the MOD-2 highway system. The collector class was discarded from further consideration in the ANOVA examination because the observed departures from normality could not be considered insignificant. Results of the normality test are given in Table A3 of Appendix A. The use of the square root transformation has a basis in theory; the data were merely a traffic occurrence or frequency distribution on the highway system, known to be distributed in a Poisson manner (Gerlough, 1955). The square root is the characteristic transformation to a normal distribution for a Poisson distributed variable.

The other ANOVA assumption tested concerned homogeneity of variance between cells. The common test for this condition is that attributed to Bartlett (1937). The square root transformed data were processed by two computer routines which yielded the chi-square values to be tested. These are given in Table A4 of Appendix A. It was apparent that cell variances of the design were quite non-homogeneous. Box (1954) has considered the variance problem also and indicates that the robust nature of the ANOVA is capable of with-standing quite a degree of heteroscedasticity.

In spite of the lack of variance homogeneity and the minor variations from normality, it was decided to continue with the ANOVA as proposed. The analysis was run separately for the two street classes with no attempt being made to examine between class effects. This decision was based principally on the variation in the number of observations between the classes. The ANOVA models took the form:

$$L_{ijkl} = \mu + P_{i} + M_{j} + T_{k} + PM_{ij} + PT_{jk} + MT_{jk} + PMT_{ijk} + \epsilon_{(ijk)l}$$

where

L ijkl purpose, by the j-th mode, in time period k, for the 1-th observation. P; is the effect of the i-th purpose, i=1,..., 3; M_i is the effect of the j-th mode, j=1,...,2; T_{k} is the effect of the k-th time period, k=1,...,2; is the experimental error; represents the link considered, 1=1,...,218, for freeways,

is the Vlink-relative-importance for the i-th

1=1,...,529, for arterials.

The interactions are similar to those defined for the first ANOVA. Computations for this analysis were performed by program BMD-2V; the results are presented and discussed in Chapter VI.

The models for the regression analysis were developed in accordance with the results of the variance analyses, and



included factors representing purpose, mode, and time. definitions of the variables and factors for regression were further modified from those used previously. The dependent regression variable (Y), in accordance with the control condition defined, was the number of trips for all purposes that were assigned to the individual links of the highway network. This represented a combination of the three purpose levels tested in the second ANOVA. The independent regression variables (X's) were similarly assigned volumes, but represented trips for a specific purpose: home-based work, non-home-based work, and home-based shop. The first two were identical to classifications in the second ANOVA; the third was an additional factor included because of the general interest and availability of the data. The shop level was not included separately in the purpose factor of the second ANOVA because the objective at that point was to define the effect of work relative to all other purposes combined. Levels of the mode factor were auto driver, identical to M_1 in both previous analyses, and highway person, a combination of the M_1 and M_2 levels of the second ANOVA. Time was treated in a similar manner: peak hour corresponded to P, and total day was the combined P_1 and P_2 levels. The definition of the regression factors closely approached the definitions of the original principal hypotheses. The only variation occurred in the second level of purpose; Po was defined as non-home-based rather than all work because of the build-up approach. Adding nonhome-based work to an equation including home-based work



implied the desired effect of total work. The factor definitions for the second ANOVA and the regression analysis are listed below together with the nomenclature for the principal hypotheses.

	HYPOTHESES	ANOVA	REGRESSION
Purpose:	HBW ALL WORK ALL PURPOSES	HBW NHBW NW	HBW NHBW ALL PURPOSES
Mode:	DRIVER PERSON	DRIVER PASSENGER	DRIVER PERSON
Time:	PEAK HOUR TOTAL DAY	PEAK HOUR NON-PEAK HOUR	PEAK HOUR TOTAL DAY
	(H:HOME, B:BAS	ED, W:WORK,	Nen;

The factor level combinations as combined for the regression variables are listed below.

Independent Variables

- 1. Home-based work, Auto driver, Total day
- 2. Non-home-based work, Auto driver, Total day
- 3. Home-based work, Highway person, Total day
- 4. Non-home-based work, Highway person, Total day
- 5. Home-based work, Auto driver, Peak hour
- 6. Non-home-based work, Auto driver, Peak hour
- 7. Home-based work, Highway person, Peak hour
- 8. Non-home-based work, Highway person, Peak hour
- 9. Home-based shop, Auto driver, Total day
- 10. Home-based shop, Highway person, Total day
- 11. Home-based shop, Auto driver, Peak hour
- 12. Home-based shop, Highway person, Peak hour

Dependent Variables

- 1. All purpose trips, Auto driver, Total day
- 2. All purpose trips, Highway person, Total day
- 3. All purpose trips, Auto driver, Peak hour
- 4. All purpose trips, Highway person, Peak hour

The requirements for regression analysis are that each value of \boldsymbol{X} be measured without error and the experimental

errors of the Y values for each value of X be normally and independently distributed with mean zero and homogeneous variance. It is not too difficult to tacitly assume that these restrictions are satisfied by the data in this analysis, particularly since there are so many possible values of X and thereby few possible values of Y for each X. The basic relationship was assumed to be linear and additive because of the nature of the urban travel situation.

The technique used was to build up a multiple linear regression equation in steps, adding one independent variable at each step. This technique follows the logic of the proposals set down for modifications in survey procedures, i.e., one variable represents one type of survey, an additional one represents a more extensive survey, etc. Thus it is shown just how much precision (increase in determination) is gained by augmenting basic survey data. The computations for the analysis were performed by computer program BMD-2R (UCLA, 1966b).

The initial run of the program permitted free selection of that variable which most significantly reduced the error sum of squares of the analysis. The respective variables added were those which caused the greatest increase in the total variation explained by their inclusion. This run eventually included all independent variables. Subsequently, a program option was exercised which selected entrance of independent variables into the equation in the logical order



of the proposed build-up survey procedure.

Nine sets of equations were developed. Four of these each had dependent and independent variables of the same mode-time combination. The mode-time combination varied between sets. The remaining five sets were developed with dependent variables common for planning and design purposes, but with independent variables of a type more easily surveyed. The latter five sets did not have similar mode-time combinations for dependent and independent variables. Within each set the dependent variable did not change. The nine equations in each set represented different combinations of the basic street classes. The first three of these treated freeways, arterials and collectors, respectively. Two more were developed by combining freeways and arterials for one (FA) and freeways, arterials and collectors for the other (FAC). The remainder included all freeway and arterial links as well as subsets of the collector group based on volume: collectors with day vehicular volume not less than 4000, 3000, 2000, and 1000. The results of the regression analyses are presented in Chapter VI and Appendix A.

CHAPTER VI. ANALYTICAL RESULTS

The analyses described in Chapter IV were designed to test the following hypotheses:

- There are no significant effects on trip volume and trip length due to the purpose, mode, and time factors and their interactions, which define trip conditions.
- 2. The pattern of travel for the work purpose adequately describes a street system sufficient to serve trips for all purposes, thereby implying feasible reorientations in travel pattern surveys.
- 3. The pattern of travel in the peak hour adequately describes a street system sufficient to serve trips made during the entire day; thereby implying a feasible reorientation in the procedure for obtaining a design hour volume.

The degree to which these hypotheses were acceptable and the ramifications of the results of the tests employed are discussed below.

The first hypothesis is concerned with a situation recognized by most practitioners. The quantitative nature of the proposed effects have been known for some time, but tests of significance have not been run. Tables Al and A2 in Appendix A are the analyses of variance for trip volume and trip



length respectively.

All main effects and interactions were significant ($\alpha=0.01$), implying rejection of the first hypothesis stated. The high significance of the main effects had been expected. It implied that the volume and length of trips observed in Indianapolis differed significantly with variation in the purpose of trip, the mode of travel, and the time of observation. The high significance of the interactions was not anticipated. It implied, for example, that the relationships between volume or trip length and the single factors (e.g., purpose) were inconsistent if any other factor was not held constant. The results of this analysis emphasized the fact that the factors being examined in regard to travel pattern development were very worthy of further consideration. They also indicated that further analyses would have to account for the interactions.

The second and third hypotheses were investigated simultaneously due to use of the same decision variable and identical statistical procedures. The second stage of the investigation involved defining the factors and models for the regression analysis. The variable was LRI and the factors examined in the ANOVA were, generally, purpose, mode and time. The LRI values were observed on the MOD-2 freeway and arterial classes only, due to the normality of their distributions. The significance level chosen for testing the F ratios was 0.25. This choice was based on the fact that probability of "type II" or " β " error (accepting a false



hypothesis) was of importance. Increasing the " α ," or probability of type I error (rejecting a true hypothesis), to the level of 0.25 reduces the probability of β error. The low β error was considered necessary because the objective of the test was to determine which effects were not significant and could thereby be eliminated from consideration in model development. The hypothesis proposed for this test was: the purpose, mode, and time factors described here have no significant effect on relative link volume. The ANOVA for these tests are shown in Tables 2 and 3. All significance tests were made using an F ratio with only the error mean square, since the model was composed completely of fixed effects.

The tests on the freeway links indicated significant results due to the main effects of time, purpose and mode. No effect on LRI was noted due to interaction. It can be concluded that LRI does vary between the peak and non-peak periods, due to change in consideration of the work or the non-work purpose, and due to travel mode. The implications are that, for freeways, separate models describing peak and non-peak traffic would yield better results than a single model. Further, there is sufficient effect due to the work purpose and mode that models describing travel must include recognition of the work purpose and means of travel. The extension of these results is valid and consistent only over the factors and levels considered here.



Table 2. Analysis of Variance for Freeway L.R.I.

Factor	Degrees of Freedom	Sum of Squares	Mean Square	F Ratio
T **	1	28,37	28.37	3.31*
M	1	17.67	17.67	2.06*
P	2	511.07	225.54	26.7*
TM	1	2.31	2.31	0.27
TP	2	3.23	1.61	0.19
MP	2	0.41	0.20	0.23
TMP	2	15.90	7.95	0.93
Error	2€04	22,318.74	8.57	~~~
Total	2615	22,897.68		

^{*} Significant at $\alpha = 0.25$

^{**} See page 105 for abbreviation key.



Table 3. Analysis of Variance for Arterial L.R.I.

Factor	Degrees of Freedom	Sum of Squares	Mean Square	F Ratio
ਜੂ* *	1	39.28	39.28	4.95*
M	1	253.22	253.22	31.9*
P	2	1,783.42	891.71	112.2*
TM	1	₹.84	3.84	0.48
TP	2	9.01	4.00	0.50
MP	2	67.98	33.99	4.38*
TMP	2	19.23	9.11	1.15
Error	6,336	50,380.69	7.95	
Total	6,347	52,554.68		

^{*}Significant at $\alpha = 0.25$

^{**}See page 105 for abbreviation key.

The tests in the ANOVA for arterials indicated the same effects observed for freeways as well as a significant mode-purpose interaction. This additional effect may reflect the change in orientation of traffic from movement to land service as street class decreases. The variability in the influence on volume exerted by work purposes cannot be considered the same for all modes, and conversely, as was the case for freeways. This implies a need for more models to account for interaction.

The hypothesis for the ANOVA of relative volumes was rejected for the purpose, mode, and time main effects on freeway links and for these as well as the mode-purpose interaction on arterial links. The remaining effects could not be rejected at Ω = 0.25. The meaning of these results must be tempered by the failure of the data to satisfy the criterion of homoscedasticity. Reflection on the trends observed gives cause for contemplation on the results which might have occurred had the collector class exhibited normality.

The equation forms used in the regression analysis reflected the results of the variance analyses. The models developed predicted trip volumes for all purposes on links of the highway network. The means of prediction were trip volumes observed on the same links for specific purposes, i.e., work. Separate models were developed for each combination of the levels of the mode and time factors. The ANOVA results indicated that the use of each additional level of

the purpose factor would increase the variation explained. Further, each mode-time combination would yield different levels of predictability, each of which was consistent within time, but not necessarily within mode. The regression equations represent the relations within the condition groups or cells of the ANOVA. The feasibility of the development approach employed was checked by permitting variables to enter the equations in a stepwise manner, according to the significance of their contribution. The final relationships were then selected, on an intuitive basis, and the respective results were compared. It should be emphasized that the following analysis was not oriented to developing predictive relationships, but rather to determining the degree to which variation in the all-purpose group was explained by variations in specific purpose groups. It is not inferred that the equations shown are applicable elsewhere, but rather that variation explained (R²) may be universal and that the respective expansion ratios (slopes) are typical.

The first regression analysis developed simple, linear relationships between peak hour and total day volumes. The equations shown in Table 4 predict total day volume based on peak hour volume. Only the all-purpose condition is presented because the objective was to demonstrate the representivity of all traffic in the peak hour. Results are shown for both modes and for three separate street classes: freeways, arterials and collectors. The variation explained (R^2) varies between modes by a maximum of 0.018; the difference in R^2 between modes increases from freeways to collectors. The prediction

4	

Table 4. Simple Regression Analysis.

Auto Driver

(F)
$*$
 Y = 824 + 5.446X R^2 = 0.969

(A)
$$Y = 899 + 5.150X$$
 $R^2 = 0.905$

(c)
$$Y = 314 + 5.307X$$
 $R^2 = 0.875$

Highway Person

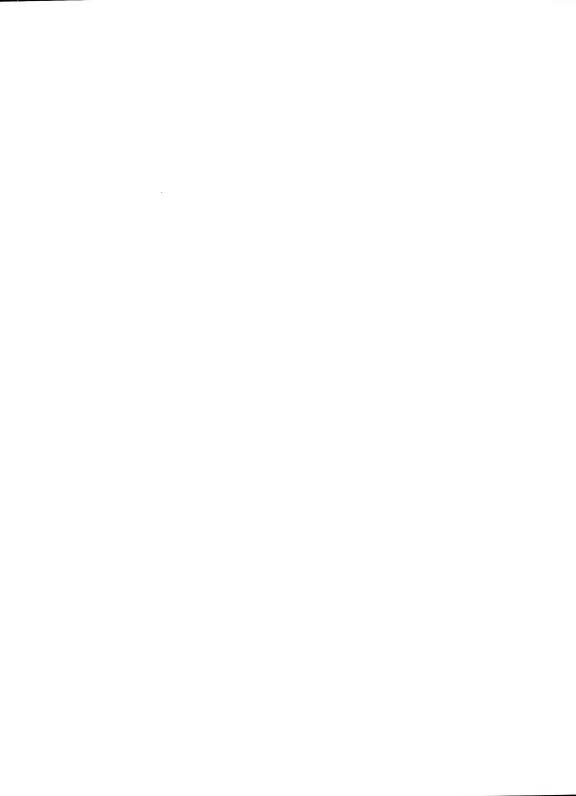
(F)
$$Y = 1549 + 5.304X$$
 $R^2 = 0.961$

(A)
$$Y = 1410 + 5.090X$$
 $R^2 = 0.895$

(c)
$$Y = 493 + 5.383X$$
 $R^2 = 0.857$

X: Total Peak Hour Trips Y: Total Day Trips

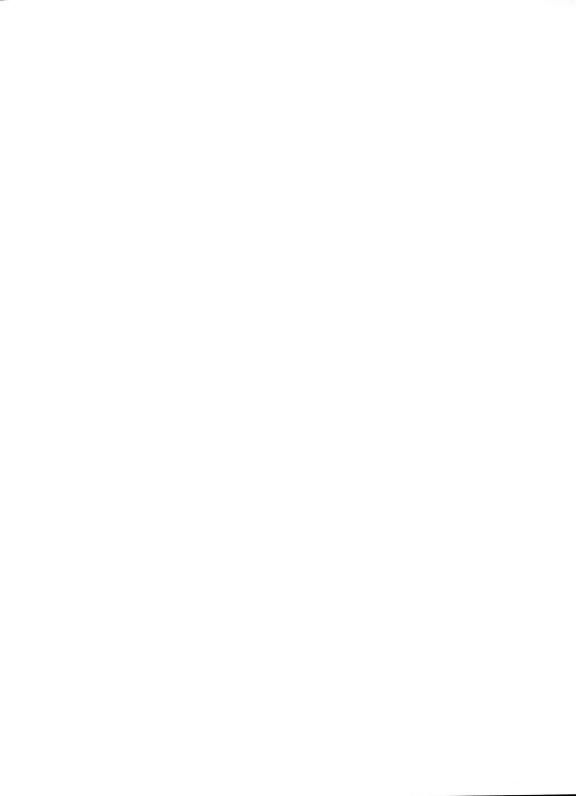
 $^{^{\}star}$ See page 105 for abbreviation key.



of total traffic based on peak hour traffic is quite reliable for all street classes. The prediction is better on higher type facilities because they are the ones most utilized in the peak hour. These positive results give credence to a basic hypothesis of this research and form a basis for further study of peak hour travel by purpose.

The multiple linear regression analyses examined the three separate street classes and several combinations of these. The examination of several street groups was undertaken in order to determine the changes in predictivity for various single classes and combinations within the total street system. Within each street group several mode-time combinations for the variables were used. In four of these cases, the mode-time condition was the same for the dependent and independent variables. In the remaining cases, the mode-time condition of the independent variable represented a more practicable survey procedure than did the mode-time combination of the predicted variable. All dependent variables represented mode-time combinations useful in planning and design.

The four homogeneous sets were concerned with predicting the total day (D) and peak hour (H) trips by highway oriented persons (P) and vehicles (V). Selection of the proper relationship would be based on the mode and time prediction desired; that same mode-time combination would be surveyed. Three of the other five relationships predicted peak hour vehicle (HV) trips using total day person (DP), total day



vehicle (DV), and peak hour person trips (HP). The results of these would be design hour traffic volumes, based on surveys of all employees, all those driving to work, and all those traveling in the peak hour respectively. The remaining two relationships used total day person trips (DP) to predict total day vehicle (DV) and peak hour person trips (HP). These represented a survey of all employees to predict daily vehicle traffic or a person trip design hour.

Tables A5 through A8 in Appendix A show results of the regressions for the free entry situation. In this situation the variables enter solution one at a time, in the order of the additional explained variation (R^2) that their inclusion will yield. The variable entering also causes the greatest reduction in the error sum of squares of all variables not in solution. The tables show the order of entry of the four most significant independent variables for each street class or combination for prediction of the four different mode-time conditions of all purpose trips. Also given are the cumulative R^2 , standard error of the estimate, and coefficient of variation at each step.

The R² values describe the proportion of variability in the predicted trips explained by the factors included in the equation at that step. Here the standard error of estimate provides the upper confidence limit on each prediction. It is indicative of the possible error that the prediction procedure may be expected to exhibit. Adding one standard error to a predicted volume assures 0.84 probability that the true volume is not greater than the result; two



standard errors implies 0.97 probability of enclosing the true value. The standard error indicates the possible error in capacity due to use of a design hour volume obtained from the procedure described here. The coefficient of variation is the proportion that the standard error is of the mean. It is an indication of relative precision of the estimate. The increase in \mathbb{R}^2 and decrease in the standard error and coefficient of variation must be balanced against the increased cost of survey necessary in order to attain these.

All independent (single purpose) variables were available for selection in the free entry regressions. The entry order and parameters of fit were affected by the individual street classes or combination of street classes considered. The entry order for a given dependent variable for the separate classes was somewhat erratic, but for the combinations it was fairly uniform. The home-based work trip that was consistent with the mode-time condition of the dependent variable was the first variable to enter in all regressions. Predictions of total day volume usually chose shopping trips second, followed by the respective non-home-based work group. The reverse was true for predicting peak hour trips. It is apparent that shopping travel is more important in total day than in peak hour travel. The fourth entry was generally too erratic to permit valid conclusions.

The ${\rm R}^2$ values were better for peak hour prediction than for total day, and better for vehicle trips than for person trips. The ${\rm R}^2$ decreased with mean volume of the street group



considered. The R² values for the FA and FAC classes were very similar, and the FA class was slightly higher in all situations but one. As more of the collector class was included with FA. the R^2 decreased slightly, until at the Tl group it began to increase. The standard errors decreased with the mean volume of the analysis group. The comparison of coefficients of variation showed an increase as more links were included in the analysis group and the group mean volume decreased. The coefficients were consistently greater for person than for vehicle trips, and were larger for total day than for peak hour trips. These results indicated which variables were most important for predicting trips of various time-mode groups. They form a basis for comparison of the results of selected variable situations. It is significant that the basic element of the proposed revised survey procedure was always the most important predictive variable.

The destination place survey procedure previously proposed implied a logical order of variable entry into prediction equations. The most elemental survey would involve home-based work trips. Next would come, in addition, non-home-based work trips. The third step would involve home-based shopping trips as well. The results discussed henceforth are for regressions on these selected variables, indicating the cumulative results (fit) as each additional variable was selected for the equation.

The regressions were run for nine street class groups and nine prediction situations within each group. Tables A9



through A16 in Appendix A show results of the regressions for all groups and situations. Tables 5, 6, 7 and 8 in this chapter show more detailed results for the FAC and FA street class combination groups. The R² and coefficient of variation values exhibited trends between street groups similar to those observed in the free entry situation. Comparisons of the results using person shopping trips versus vehicle shopping trips indicated that the vehicle trips usually explained more variation than the person group. As a result the vehicle mode was used for all home-based shop variables included in the analysis. Use of the vehicle mode permits a license plate survey of shop trips.

When using the same mode-time condition for both dependent and independent variables, the FAC group (all street system links) was second in prediction (R²) only to the free-way class (Table A9). The two groups were very close in all cases. The difference in R² between separate street classes was greater than between progressive groups combining collectors with the FA group. The standard errors of the estimates decrease as more of the collector links are included with those in the FA group (Table A10). The FAC group exhibits the lowest standard errors in most situations. The coefficients of variation were less (Table A11). If precision is important, the FA group is better than the FAC, since the coefficients of variation are less. If capacity error is more important, the FAC group is better, since the standard errors are less. A good balance in this decision

Parameters and Coefficients * Multiple Linear Regression Selected Predictors Normal Dependent Conditions Entire Street System (FAC). Table 5.

			Parameters	eters			Coef	Coefficients	
X	×	R ²	ΔR^2	ម	C.V.	L) 0	В	B ₂	B 3
DV	1 2 13	0.9307 0.9573 0.9875	0.0266	717 563 304	0.249 0.196 0.106	620 ?36 123	1.498 1.290 1.140	1.587	 1.615
DP	4 S E	0.9076 0.9337 0.9792	0.0261	1188 1007 565	0.280 0.237 0.133	1083 682 294	1.601 1.379 1.183	1,986	2.820
НV	7 8 7	0.9510 0.9791 0.9911	0.0281	100 69 45	0.231	77 31 14	1.185	1.271	1.330
НЪ	10 11 15	0.9442 0.9719 0.9866	0.0277	161 114 79	0.244 0.17? 0.120	126 59 31	1.227	1.486	2.106

See page 105 for abbreviation key.



Parameters and Coefficients $\hat{\mbox{Multiple}}$ Linear Regression Selected Predictors Special Dependent Conditions Entire Street System (FAC). Table 6.

		٠	· · · · · · · · · · · · · · · · · · ·							
				Parameters	eters			Coef	Coefficients	
> -	×		R ²	ΔR^2	S.E.	C.V.	ВО	Bl	B2	B3
		4	9196	1	773	990 0	αιν	1 124		
DΩ	DP	· ഹ	0.9408	0.0211	663	0.231	406	0.985	1,246	1
		13	0.9765	0.0357	418	0.145	167	0.864	1,163	1.742
		4	0.9287	!	182	0.276	103	0.282	1]
HP	DP	Ŋ	0.9470	0,0183	157	0.238	44	0.250	0.290	1
		13	0.9577	0.0107	140	0.212	11	0.233	0.279	0.239
		4	0.9193	!	135	0.297	69	0.196	I	1
HV	DP	Ŋ	0.9327	0.0134	123	0.270	34	0.176	0.173	!
		13	0.9423	9600.0	114	0.251	13	0.165	0.165	0.157
		10	0.9353	!	121	0.266	85	0.850	1	}
HV	HP	11	0.9584	0.0231	97	0.213	42	0.815	0.943	1
		15	0.9737	0.0170	77	0.169	22	0.763	0.898	1,495
		~	0.9341	!	122	0.268	62	0.259	1	1
HV	DV	7	0.9520	0.0179	104	0.228	21	0.231	0.226	1
		13	0.9586	0.0066	96	0.211	4	0.219	0.218	0.132

* See page 105 for abbreviation key.



Parameters and Coefficients * Multiple Linear Regression Selected Predictors Normal Predicted Situations Freeway and Arterial Classes (FA). Table 7.

0.9266 919 0.9266 919 0.9524 0.0258 741 0.9868 0.0344 391 0.9305 0.0262 1273 0.9793 0.0262 1273 0.9793 0.0278 88 0.9799 0.0278 88 0.9719 0.0112 58								
1 0.9266 919 2 0.9524 0.0258 741 13 0.9869 0.0344 391 4 0.9043 1492 5 0.9305 0.0262 1273 13 0.9793 0.0488 695 7 0.9561 129 8 0.9799 0.0278 R8 15 0.9911 0.0112 58	0	ΔR^2	м П	C.V.	ВО	В	B ₂	ВЗ
2 0.9524 0.0258 741 13 0.9869 0.0344 391 4 0.9043 1492 5 0.9305 0.0262 1273 13 0.9793 0.0488 695 7 0.9561 129 8 0.9799 0.0278 R8 15 0.9911 0.0112 58	,		919	0.167	1183	1,382	!	1
4 0.9043 1492 5 0.9305 0.0262 1273 13 0.9793 0.0488 695 7 0.9561 129 8 0.9799 0.0278 R8 15 0.9911 0.0112 58	00	0.0258	741	0.135	715 314	1.220	1.611 1.456	1.513
5 0.9305 0.0262 1273 13 0.9793 0.0488 695 7 0.9561 129 8 0.9799 0.0239 88 15 0.9911 0.0112 58	0.9043	1	1492	0.187	2047	1,465	ţ	1
13 0.9793 0.0488 695 7 0.9561 129 8 0.9799 0.0278 88 15 0.9911 0.0112 58	0,9305	0.0262	1273	0.159	1391	1,275	2.057	1
7 0.9561 129 8 0.9799 0.0238 A8 15 0.9911 0.0112 58	0.9793	0.0488	695	0.087	689	1.158	1.827	2,555
8 0.9799 0.0278 R8 15 0.9911 0.0112 58	0,9561	!	129	0.146	147	1,131	ŧ	1
0.9911 0.0112 58	0,9799	0.0278	80 α	0,100	62	1,116	1.222	1
195	0.9911	0.0112	53	0.066	36	1,064	1,140	1.248
	0,9519	i	195	0.154	231	1,167	***	-
HP 11 0,9737 0,0217 145 0	0,9737	0.0217	145	0.114	111	1,151	1.401	1
0.9869 0.0172 102	0,9869	0.0132	102	0.080	29	1.092	1,321	1,955

*See page 105 for abbreviation key.

Parameters and Coefficients * Multiple Linear Regression Selected Predictors Special Predicted Situations Freeway and Arterial Classes (FA). Table 8.

		2	500000000000000000000000000000000000000	- 16	,	1 1 C C C C C C C C C C C C C C C C C C	ALCCE TO T	n (0	(1 11)	
				Parameters	sters			ဝဝ၁	Coefficients	
⊱	×		R ²	Δ _R ²	о	C.V.	ш О	В	B ₂	B ₃
		4	0.9119	!!	1007	0.183	1299	1.035	!	!
DV	D P	13	0.9316 0.9705	0.0197	07 08 08 08 08 08 08 08 08 08 08 08 08 08	0.162	898 458	0.918 0.845	1.257	1.603
		4	0.9257	į	243	0.191	157	0.274	i	1
НР	D D	13	0.9438	0.0181	211 192	0.166	- 56	0.245	0.316	0.214
		4	0.9084	1	187	0,212	118	0.118	1	i i
НΛ	OP	13	0.9206 0.9288	0.0122	174 165	0.197	61 24	0.171	0.180 0.168	0.134
HV	НР	10	0.9384	0.0172	153	0.173	168 93	0.803	0.864	; ;
		15	0.9687	0.0130	110	0,125	63	0.753	0.809	1,342
,	į	Н (0.9314	1 1	162	0.184	94	0.252	1	!
AV.	DA	13	0.9486 0.9545	0.0172	140	0.159	24 - 6	0.228	0.240	0.114
					-					

 $^{\star}_{\text{See page 105}}$ for abbreviation key.



is desirable, but standard errors are more readily translatable into effects, i.e., design errors.

The regression coefficients shown in Tables 5 through 8 are related to the proportion that the several purpose groups are of total travel. The regression equation constants are indicative of the travel attributable to purposes not accounted for by the variables in the equation. The current research proposes no more than that these coefficients may be generally applicable. Their validity must be further tested to verify this premise.

The regressions involving dependent and independent variables of different mode-time groups are summarized in Tables Al2 to Al5 of Appendix A. The precision exhibited was not as good as in the homogeneous cases. The differences were slight in all cases, however, and probably easily rationalized in light of survey cost savings. The general trends of all parameters are similar to those observed in the homogeneous cases. Some discrepancies which are unexplainable may probably be best attributed to anomalies of the data or analysis. The important fact is that the generally excellent predictivity is exhibited in these situations which imply more feasible survey procedures.

The implications of these results are important to the selection of survey procedures. Based on the increase in \mathbb{R}^2 and the decrease in standard error of estimate and coefficient of variation, the order of survey complexity follows directly.

The same progression follows for all mode and time groups. A home-based work, vehicle trip survey requires employer records of those employees driving to work. A home-based work person trip survey requires tabulation of all employee records and a tabulation regarding mode. An all work trip (home- and non-home-based) survey requires interview of drivers or all employees, depending on the mode of interest. Extension to include shopping trips requires, in addition, a license plate survey at shopping districts. Decisions on the form of any revised survey procedure are best made according to costs and feasibility. This research has provided the study director with alternative procedures for replacing the costly home-interview survey. It must be his decision, in light of the community conditions, to select a feasible alternative which will provide valid travel patterns at the least possible cost.

CHAPTER VII. APPLICATION OF THE METHOD

The techniques discussed in the preceding chapters were proposed to eliminate the need for a home-interview survey to establish the patterns of urban travel. It is the purpose of this chapter to suggest how the techniques proposed from the results of Chapter VI would be implemented in the urban transportation planning process. It is important to carry the results of the revised survey procedure through the stages of the planning process in order to demonstrate the revisions which must be made therein to account for the different data.

manner, defining the study area, zones and the transportation networks. The land use survey required would be limited for all classes except residential to defining the location, size of plot and extent of development. Since work is the only purpose group to be surveyed and employment is the generation parameter for work trips, there is no need to collect conventional land use data. Employment will be obtained at the time of the workplace contact. The economic base and population studies would be executed as usual. It is possible that further refinements of the proposed approach might require more detail from these socio-economic surveys. The



truck-taxi surveys would be executed in typical fashion. The external survey would be performed as usual but would eliminate all internal terminating trips for non-work purposes.

The major revisions concern the home-interview survey. If an all-purpose, peak hour analysis has been selected, the only revision will be elimination from the home-interview data of all but peak hour (as defined by the user) trips. Use of a work trip oriented approach would imply complete elimination of the home-interview. Further decisions on procedure rest on the extent of analysis desired. A home-based work oriented analysis requires only examination of employer records. basic information to be obtained is the address of the employee's residence. Additional information regarding salary and family characteristics might also be obtained to refine the analysis and do special studies, providing necessary permission can be obtained. If the analysis is to consider trips during the entire day, all employees would be tabulated. A peak hour orientation would require definition of the shift or reporting (or leaving) time of the workers, selecting only those which would necessarily be traveling during the peak hour chosen. Use of vehicle trips only, or a need to determine mode for all travelers would employ a procedure such as having employees place their time cards in different bins, according to their travel mode. An alternative approach would involve mode tabulation for each worker by either supervisors or survey personnel.

Decision to include all work trips in the analysis would require a more involved survey. In essence, each worker would have to be interviewed in some manner. The interviews could be conducted one at a time by either supervisors or study staff personnel. Alternatively the workers could be surveyed by use of a questionnaire. Direction in completion of the questionnaire could be given either locally by monitors or to the entire work force simultaneously by means of a public address system. The basic data sought would be the address of the origin and destination of all trips made to or from the employment place. A "contact" survey could also obtain such information as mode, time, and routing of the work trip as well as attitudes regarding work travel. Use of a questionnaire is less satisfactory than an interview, but it is also less expensive and time consuming (Wohl, et al., 1959). The balance in difficulty and cost would have to be weighed against utility. Further extension of the proposed procedures to include shopping travel would involve tabulation of arrivals at major (defined by the analyst) shopping areas. Use of only home-based shopping trips by vehicles would merely require tabulation of license plates. This could include the usual parking study as an adjunct. Including all home-based shopping person trips would require tabulation of the number of persons arriving in each vehicle. Shopping trips by transit would be prohibitively difficult to survey.

A comparison of costs to obtain data would be of value at this point. Table 9 indicates the relative cost in hours of three approaches being discussed. The figures are based on a coding procedure developed at Purdue that would be an appropriate approach to handling employee records only (Zimmerman, 1967). The rate for employment place interviews was estimated at 20 per hour. One last note would be in regard to another alternative to the above proposed technique. Use of a city directory could provide similar information at a further cost reduction (Udy, 1962).

Depending on the survey procedure and approach selected, expansion of the survey results might be necessary in order to obtain true traffic volumes. The expansion could be made according to typical figures which have been shown to be quite stable and reliable (see Lapin, Chapter II). Alternatively the expansion could be based on figures from microsample home-interviews or questionnaires sent home with workers interviewed in the principal survey. The regression coefficients developed in the previous analysis may provide at least a good first approximation to expansion factors. The results of the survey or the expansion, as applicable, would be assigned to the street network in the same manner as usual, followed by the appropriate capacity restraint iterations. Development of the distribution model would use work and residential trip ends developed from the survey in the same manner as usually employed. The model results would be expanded by the traffic volume factors discussed previously



Table 9. Relative Cost in Hours of Three Alternative Survey Procedures.

	Lafayette	e Indianapolis
Coding Employee Records (60 per hour)	353	5,627
Employee Interviews (20 per hour)	1,059	16,880
Home-Interview (5 percent sample) (1,942 hours each)*	1,641	20,598

^{*}Based on Data from Austin, Texas, Transportation Study, Published by the Texas Highway Department.



in order to perform screenline and cordon checks where appropriate.

Development of the future travel potential would, of course, be based on the economic base and population studies. These would be combined with the existing land use information to determine the future pattern of employment generating activities. After allocating employment and residential activities, the work trips would be distributed as before. The traffic expansion factors, modified to account for changes over time, are then used to obtain total volumes. These are assigned to the committed network and the assignments are adjusted as necessary by capacity restraint. The deficiencies are then taken into consideration in developing the new facilities of the target year transportation system.



CHAPTER VIII. CONCLUSIONS

From the previous analysis, several conclusions can be drawn regarding the proposed hypotheses and related relationships:

- The variance analysis proved to be of considerable value in development of the regression relationships.
- 2. The travel patterns of home-based work trips represent an excellent approximation to the travel patterns of trips of all purposes for peak hour or total daily travel.
- Use of all work trips improves the above relationship slightly.
- Use of shopping trips in addition to work trips further improves the representivity slightly.
- 5. The relationships discussed above were appropriate for travel by all highway oriented modes. Use of highway person trips showed marginally better results than auto driver trips.
- 6. The improved results observed in moving from statement 2 to statement 5 are due to the increased information obtained from the survey. The marginal improvements must be weighed against the increased cost due to the more extensive survey procedure.

- 7. Travel for all purposes during a peak hour showed excellent results in representing travel for all purposes during the entire day.
- 9. Total day travel for the work purpose was a good representative of the peak hour travel by all purposes.
- Typical results of the regression analyses performed on IRTADS data are shown below.
 - a. Predicting all purpose travel using all work and home-based shopping trips, for peak hour vehicle travel on the entire highway system:

 $R^2 = 0.9911$ S.E. = 45 C.V. = 0.099

b. Predicting all purpose travel using all work and home-based shopping trips, for the total day vehicle travel on the entire highway system:

 $R^2 = 0.9875$ S.E. = 304 C.V. = 0.106

c. Predicting all purpose peak hour vehicle travel using all work and home-based shopping person trips for the total day on the entire highway system:

 $R^2 = 0.942$ ° S.E. = 114 C.V. = 0.250

d. Predicting total day vehicle travel for all purposes using total day person trips for all work and home-based shopping purposes, on the freeway and arterial classes:

 $R^2 = 0.9705$ S.E. = 583 C.V. = 0.106

10. The data used in these analyses were samples from all work trips. The proposal survey procedures would include only trips to major employment concentrations.



CHAPTER IX. RECOMMENDED EXTENSIONS AND RESEARCH

Based on the preceding analyses and conclusions, the following further study is recommended:

- The regression coefficients of the prediction equations established for the IRTADS data should be tested on data from other urban areas. If no significant differences are found, the survey expansion procedure would be greatly simplified.
- 2. The representivity of the work trip for all purpose travel should be examined in different size urban areas in order to define the effect of size on representivity.
- 3. The surveys proposed in this research should be executed simultaneously with a traditional travel survey in order to define the relative strengths of each approach.
- of characteristics of the place and type of employment on the observed relationships in order to improve the estimates of work trip generation. This seems appropriate because of the importance that work travel will assume in prediction of total urban travel.



- 5. Study should be made of the effect the number of the employees in each work place has on the observed relationship in order to better define the extent of survey required.
- 6. More detailed investigation should be made of the implications of various types of industrial and residential land use locations since this will become highly important in planning based on the procedure proposed here.
- 7. A study should be made of the effect on prediction of total travel by using only those trips to major employment concentrations. These effects should be established for urban areas of varying size.





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APPENDIX A

Abbreviation Key

Dependent Variables (Y)

DV	Total Day Vehicle Trips
DP	Total Day Person Trips
HV	Peak Hour Vehicle Trips
HP	Peak Hour Person Trips
	Independent Variables (X)
1	Home-Based Work, Vehicle, Total Day
2	Non-Home-Based Work, Vehicle, Total Day
4	Home-Based Work, Person, Total Day
5	Non-Home-Based Work, Person, Total Day
7	Home-Based Work, Vehicle, Peak Hour
8	Non-Home-Based Work, Vehicle, Peak Hour
10	Home-Based Work, Person, Peak Hour
11	Non-Home Based Work, Person, Peak Hour
13	Home-Based Shop, Vehicle, Total Day
14	Home-Based Shop, Person, Total Day
15	Home-Based Shop, Vehicle, Peak Hour
16	Home-Based Shop, Person, Peak Hour
F	Freeways
А	Arterials
С	Collectors



FA	Combined Freeways and Arterials
FAC	Combined Freeways, Arterials and Collectors
Т4	Freeways, Arterials and Collectors with volume greater than 4000
Т3	Freeways, Arterials and Collectors with volume greater than 3000
T2	Freeways, Arterials and Collectors with volume greater than 2000 $$
Tl	Freeways, Arterials and Collectors with volume greater than 1000
Н	Home
N	Non
В	Based
W	Work
S	Shop
PAS	Passenger
OTH	Other
D	Total Day
H	Peak Hour
V	Vehicle
P	Highway Person
R ²	Explained variation
Δ R 2	Increase in Explained variation
S.E.	Standard Error of the estimate
C.V.	Coefficient of variation
Во	Regression Equation Constant
В1	Regression Coefficient of First Variable
B ₂	Regression Coefficient of Second Variable
B ₃	Regression Coefficient of Third Variable

Table Al. Analysis of Variance for Travel Volume.

Factor	Degrees of Freedom	Sum of Squares	Mean Square	F Ratio
M T P MT MP TP MTP Error	2 23 5 46 10 115 230 1,296	390,696.9 492,524.0 467,009.9 227,956.2 585,531.9 1,057,252.5 752,835.5 28,157.0	195,348,4 21,414.1 93,402.0 4,955.6 58,553.2 9,193.5 2,273.2 21.7	* * * * * * * *

^{*} F ratios are not shown because of the obvious significance of every factor and interaction.



Table A2. Analysis of Variance for Average Trip Length.

Factor	Degrees of Freedom	Sum of Squares	Mean Square	F Ratio
M T	2 23	4,388.9 15.708.4	2,194.4 683.0	+ *
P	23 5	10.766.2	2,153.2	*
MP	46	5,261.0	114.4	*
MP	10	1,731.7	173.2	*
TP	115	9,045.9	78.7	*
MPP Error	230 1,296	10,649.1 29,177.6	46.3 22.5	*
51101			60 60 6 5	
Total	1,727	86,728.9		

 $^{^{\}star}\mathrm{F}$ ratios are not shown because of the obvious significance of every factor and interaction.



Table A3. Kolmogorov-Smirnov Test for Normality Maximum Differences for MOD-2 L.R.I.**

	Freeways	Arterials	Collectors
HBW-V-D*** NHBW-V-D OTHER-V-D HBW-P-D NHBW-P-D OTHER-P-D HBW-V-H NHBW-V-H OTHER-V-H MDW-P-H NMBW-P-H NMBW-P-H OTHER-P-H	0.0845 0.1185* 0.0448 0.1024 0.0688 0.0313 0.0974 0.09€1 0.0670 0.1061 0.1957* 0.0348	0.0571 0.0296 0.0371 0.0621 0.0911* 0.0443 0.0538 0.0624 0.0336 0.0546 0.2012* 0.0458	0.0475* 0.0772* 0.0641* 0.0490* 0.4398* 0.0420* 0.0812* 0.1770* 0.1016*
SAMPLE	218	529	1793

^{*} Significant at $\alpha = 0.01$.

^{**} Tests made on square root transformed data.

^{***}See page 105 for abbreviation key.



Bartlett's Test for Variance Homogeneity Chi-Square for MOD-2 L.R.I. Table A4.

		Freeway *	Arterial *	Degrees of Freedom
Between All Cells		739.98	245.91	11
Retween Cells Within Time	Day Peak	129.69	69.17	n n
Between Cells Within Purpose	HBW NHBW	14.69	5.21	ო ო ო
	OF DEA) + •	•	

All values were significant at $\alpha = 0.01$.

** Tests made on square root transformed data.

Significance Order and Related Parameters * Multiple Linear Regression Prediction of Total Day Vehicle Irips (DV) All Street Classes and Combinations. Table A5.

- 1	arceron.	מי די סכמי די	7				2000	
\Z		O	FA	FT 4	ŁΤ	Т2	T1	FAC
	i	Va	riables E	ntered in O	rder (X)			
14 12		1221	13	122	4 4 6	13	13	13
13		\vdash	4	4			4	
		Cumulativ	e Proporti	on Variati	on Explain	ed (R ²)	! ! !	! ! !
. 861 936		α α α	92	916	.910	911	920	930
0.9727		0.9631	0.9869	0.9840	0.9820	0.9821	0.9843	0.9875
1 1		Cumulati	ve Standar	d Error of	Estimate	(S,E.)	1 1	1 1 1
951		656	616	006	- 60 (824	0	717
645 423		43/250	291	625 395	389	605 371	565 341	304
397		247	381	α) α, α.	Œ.	365	(1)	299
1 1		Cumulat	ive Coeffi	cient of V	riation	(C,V.)	1 1 1	1 1 1
.17		C. 1	0.167	16	-	.18	. 20	.24
• 12		. 24	-	.11	.12	T3	-	.17
0.079		0,140	07	0.074	0.079	0.084	0.092	0.106
\ \		Ÿ ⊣	90.))		, ,)

* See page 105 for abbreviation key.

Significance Order and Related Parameters * Multiple Linear Regression Pre-Table A6.

	diction	of Total I	Day Person	Trips (DP)	All Stree	t Classes	and Combin	Combinations.
Ĺτ4	A	υ	FA	₽ ₽	€ 3	T2	TJ	FAC
		Va	riables E	ntered in O	rder (X)			
13	14	134	14	14	7 4	441	7 4	14
ω -	ſΩ	ы c	ν·-	ഥെ	ω -	ر د د	ر د د	ا ا
7	4	4	-1			n H		7
1 1 1 1	! ! ! !	Cumulativ	ve Proporti	on Variati	on Explain	(\mathbb{R}^2)	 	
.961	.961	756	904	. 092	984	884	.894	
985	.921	P67	960	.950	.946	. 944	.949	
0.9912	0.9600	0.9288	0.9709	0.9745	0.9713	0.9708	0.9733	0.9781
.991	. 963	თ ო თ	980	. 975	. 972	. 972	. 974	
		Cumulati	ive Standar	rd Error of	Estimate	(S.E.)	1 1	1 1 1
\sim	1525		149	10		1340	1262	α 0
α σ.	994	669	960	954	951	925	874	810
5	713	\sim	0	$\overline{}$	\circ	674	635	7
4	681	472	[ω	(I)	629	809	9
1 1	1 1	1	1 9	1 4	1 4	- 1	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	1 1 1
		C Camar 34	ive coeir	iclent of	ariarion	7. ^ - 7		
0,160	0.197	0.353	791.0	0.188	0.194	0.206	0.232	0.280
0 0	. 12	. 26	-	⊢.	4	14	۲.	٦.
07	0	. 17	\circ	0	.09	.10	۲.	⊣.
07	0	.17	\circ	O.	0	. 10	۲.	⊢.
*								
000	14	777	1.00					

See page 105 for abbreviation key.



Significance Order and Related Parameters * Multiple Linear Regression Pre-Table A7.

	diction	Of Peak T	our Vehicle	e Trips (HV	All Str	eet Classes	and Combinati	nations.
ĹΉ	A	U	FA	£-	T3	Z.L	Tù	FAC
		lεΔ	ariables En	tered in	Order (X)			
111	С α ги ги	να 21 21 13	7 11 11	7 8 15	2 1 11	7 11	7 2 1 11	7 8 7 1 3 2 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3
1 1	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	Cumulativ	ve Proporti	ion Variati	on Explain	ed (R ²)	1 1 1 1 1	1 1 1
0.9869 0.9928 0.9960 0.9961	0.9625 0.9845 0.9845	a) 0, 0, 0	0.9561 0.9799 0.9911 0.9914	0.0493	0000	$\sigma \sigma \sigma \sigma \sigma$	0.9450 0.9750 0.9893 0.9893	4044
1	1 1	Cumulati	ive Standar	rd Error of	Estimate	(S, H, S)	; ; ;	f i f
101 75 56 55	137 922 539 53	37 57 75 50	<u>и</u> с с с с с с с с с с с с с с с с с с с	128 728 578 57	126 84 73 56	121 83 54	114 77 50 50	105 69 44 44
1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	1	Cumulat	tive Coeffi	cient of	Variation ((C.V.)	1 !!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!	1 1
0.110 0.032 0.061 0.060	0.158 0.106 0.068 0.067	0.314 0.206 0.134 0.130	0.146 0.100 0.066 0.066	0.150 0.102 0.068 0.067	0.159 0.106 0.072 0.070	0.170 0.117 0.076 0.076	0.194 0.131 0.085 0.085	0.231 0.152 0.099 0.097
* See page	e 105 for	abbreviat	ion key.					

on Pre- nations.	FAC		10	15		1 1 1	944	971	0.9866	987	1 1	161	114	79	77	1 1 1		24	0.173	1 -	
Regressio	I		10	121	-	1 1 1	937	. 967	0.9843	985	1 1	1	126	87	80	1 1		. 20	0.148		•
ple Linear t Classes	1,2		10	15	m 	$ed(R^2)$	934	. 964	0.9832	00	(S E)	00)	136	94	92	1	(.7.)	Τ.	0.133		
ers Multi All Stree	W E-4	rder (X)	01	157	⊣	on Explain			9886.0	•	Estimate	9	139	97	96	1	riation (,16	0.121	ο α Ο C)
ed Paramet(Trips (HP)	T.4	sered in Or		15		on Variatio			0.9047		d Error of	9	144	\circ	0	I I I I	clent of V	.15	0.117	ο α ο α	•
and Relate	FA	riables Ent		ਮ ਹ ਜ		Froporti	951	0.9737	0.9069	0.9875	ve Standar	0	145	\circ	0	1 1 1 1	ive Coeffic	\vdash	0.114) [
Order	U	V = 1	010	15	23	Cumultive	0.0705	5	0.9610	9	Cumulativ	136	96	65	62	1 1 1 1 1	Cumulat	.33	0.236	1 – 5 FC	1
Significance diction of Pe	A		10	12	e H	1 1 1	. 901	.944	0.9751	976	1 1 1	0	155	0	0	1 1 1 1		.16	0.125	0 α ο C	•
Table A8.	Ĺ		10	16	7	1 1 1 1	. 985	. 992	0.9955	966.	1 1	154	112	78	85	! ! !		11	0.084		

See page 105 for abbreviation key.

Table	A9.	Proportion o Predictors N	of Explained Variation Normal Prediction Situ	(R') ation	Multiple Linear F s All Street Class	Regression Sesses and Combir	on Selected Combinations.
		Home-Based	Work	Non-Home-Base	ed Work	Home-Based	Shop
		d	\sim	Дı	Λ	C.	Λ
ĹΉ	ΔΞ	0.9616 0.9859	0,579 0,986,0	0.9702 0.9925	0.9916 0.9926	0.9912 0.9949	0.9943 0.9959
Ø	ΔĦ	0.8160 0.9013	0.8613 0.9157	0.8692 0.9448	0.9104 0.9625	0.9602 0.9945	0.9759
O	QH	0.7566 0.8305	0.8108 0.8546	0.9149	0.8866 0.9374	0.9388 0.9610	0.9631
FA	ДН	0.9043	0.9266 0.9561	0.9305	0.9524 0.9799	0.9793 0.9869	0.9868
T.4	ДЩ	0.8927 0.9444	0.9168 0.9493	0.9191 0.9693	0.9443 0.9764	0.9749 0.9847	0.9840 0.9895
Т3	ДΗ	0.8840 0.9373	0.9105 0.9434	0.9107 0.9664	0.9393 0.9738	0.9721 0.9836	0.9820 0.9885
12	ДΗ	0.8844 0.9344	0.9116 0.9412	0.9111 0.9644	0.9407 0.9724	0.9718 0.9832	0.9821 0.9882
TI	QH	0.8946 0.9379	0.9205 0.9450	0.9206 0.9671	0.9483 0.9750	0.9744 0.9843	0.9843 0.9893
FAC	ДΉ	0.9076	0.9307 0.9510	0.9337	0.9573 0.9791	0.9792 0.9866	0.9875

for abbreviation key. *See page 105



* Selected Pre-Prediction Situations All Street Classes and Combinations. Shop 58 >Home-Based 93 65 Standard Error of Estimate (S.E.) Multiple Linear Regression D, Non-Home-Based Work 88 85 69 > 144 96 126 a, Work 126 \triangleright dictors Normal Home-Dased 136 Д Pable AlO. OH OH a E O H ΩH ΩH ρH ΩI Q II FAC FA 7.5 T I 14 (14 Þ U

See page 105 for abbreviation key.

0.106 Shop 0.074 0.140 071 066 0.074 0.079 0.084 0.092 Selected Pre-Combinations 0.062 \triangleright 00 eq ú Ø Home-E 0.092 0.091 0.096 0.077 177 160 0.087 0.102 0.114 0.133 D. and 0 0 Regression Classes and Linear Street Work 0.245 0.139 0.168 m w 0.135 0.144 0.152 0.196 m (r 0.113 0.143 sed \triangleright Coefficient of Variation (C.V.) Multiple dictors Normal Prediction Situations All -Has Non-Home 0.141 166 125 159 201 .237 301 236 164 171 400 \vec{H} 00 00 00 00 00 00 00 00 Work 0.231 209 169 169 175 186 137 $\alpha : \alpha$ 167 146 31 2 \gt 5 \neg · · · 00 00 00 00 00 00 00 sed ſŪ Home-I 0.188 0.206 353 0.160 0.197 0.197 0.194 0.166 0.232 0.280 Д 00 H Al OH OH OH OH OH Q H DI OI QH able FAC (~) 2 1 EA \mathbb{T}_4 Щ Q U \vdash

See page 105 for abbreviation key.

Proportion of Variation Explained (R 2) Multiple Linear Regression * Prediction of Total Day Vehicle Trips (D^*) Peak Hour Person Trips (HP) Based on Shop 0.9409 0.8866 0.9463 0.9772 0.9211 0.9475 0.9577 Home-Based and Combinations 0.9459 0.9442 0.9705 0.9663 0.9643 0.9700 0.9377 .206 0 DV Classes Non-Home- sed Work 0.9752 0.8968 0.9518 0.9352 0.9272 7566 0 0.9470 HP Street Total Day Person Trips (DF) All C#36 0 0.9773 0.9697 0.9542 0.9710 0.9219 0.9040 20 o29a.∪ 0.3044 Work 0000.0 0.9257 0.9287 0.9691 0.9147 0.9157 0.3056 d H Home-Based 0.8988 0.7952 0.9119 0.9025 0.9665 0.8716 0.8974 0.9077 0.8761 M A12. Table FAC 74 8 H FA A.

See page 105 for abbreviation key.

Standard Error of Estimate (S.E.) Multiple Linear Pegression * Prediction of Total Day Vehicle Trips (DV) Peak Hour Person Trips (HP) Based on Total Day Person Trips (DP) All Street Classes and Combinations. Table Al3.

	Home-Jased Work	SEC WORK	N 70 - 200 - 200 - 1001			doile sases oile
	DV	HP	DV	НР	DV	HP
ſτι	8 8 8	227	796	203	540	196
A	1043	245	£ 26	212	596	186
O	588	C. 44	496	127	705	111
FA	1007	7 70	დ ლ ლ	211	593	192
T4	974	0#2	872	600	573	190
E L	927	- - - - -	₩ 60	205	547	185
T.2	33]	221	789	193	514	174
71	829	202	730	7-1	472	160
FAC	096	132	945	157	691	140

* See page 105 for abbreviation key.

Coefficient of Variation (C.V.) Multiple Linear Regression * Prediction of Total Day Velicle Trips (DV) Feak Hour Person Trips (HP) Based on Total Day Person Trips (DP) All Street Classes and Combinations. Shop 0.273 0.170 0.188 0.148 0.150 0,151 0,162 0.154 0.212 HЪ Home-Based 0.172 0.106 0.108 0.116 0.12§ 0.093 0.237 D Non-Home-Based Work 0.166 0.418 0.209 0.157 0.170 0.170 0,100 0.238 HP 0.172 0.278 0.160 0.164 0.294 0.16. 0.176 Work 0.137 0.171 0.135 0.263 0.191 0.105 0.216 0.237 0.276 ΗЪ Home-Jased 0.330 0.152 0.107 0,191 0.224 0.334 0.183 0.182 0.193 2 Table A14. FAC FA 4 7.5 I O Q, Ĺίι

See page 105 for abbreviation $\mathrm{ke}\gamma$.



Table Al5. Proportion of Variation Explained (R²) Multiple Linear Regression Prediction of Peak Hour Vehicle Trips (HV) Three Special Prediction Situations All Street Classes and Combinations.

		HBM	NFEW	HBS
F	DP	0.9607	0.9634	0.9658
	HP	0.9904	0.9857	0.9896
	DV	0.9732	0.9764	0.9779
А	DP	0.8378	0.8617	0.3799
	HP	0.8797	0.9137	0.9413
	DV	0.87€1	0.9104	0.9232
С	DP	0.7956	0.3 ² 4 ³	0.9665
	HP	0.8214	0.8904	0.9400
	DV	0.8110	0.8643	0.8884
FA	DP	0.9084	0.9206	0.9298
	HP	0.9394	n.9557	0.9697
	DV	0.9314	n.9486	0.9545
T4	DP	0.8959	0.9092	0.9190
	HP	0.921	0.9599	0.9656
	DV	0.9201	0.2499	0.9485
ئى ئ	DP	0.9889	0.3021	0.9130
	HP	0.3066	0.9474	0.9643
	DV	0.9109	0.9329	0.9412
T2	DP	0.9913	0.3054	0.9166
	HP	0.9243	0.9465	0.9650
	DV	0.9120	0.3533	0.9423
71	DV DP	0.9039 0.3070 0.9217	0.9174 0.9502 0.9412	0.9279 0.9678 0.9487
FAC	DP	0.9193	0.9327	0.9423
	HP	0.9353	0.9594	0.9797
	DV	0.9341	0.9520	0.9586

 $^{^{\}star}$ See page $_{105}$ for abbreviation key.



Table Al6. Standard Error of Estimate (S.E.) Coefficient of Variation (C.V.) Multiple Linear Regression Prediction of Peak Hour Vehicle Trips (HV) Three Special Prediction Situations All Street Classes and Combinations.

		Sta	ndərd Er (S.E.)	ror	Coeffic	cient of (C.V.)	Variation
		HBW	NHBW	HBS	HBW	NHBW	HBS
F	DP	174	168	163	0.189	0.183	0.177
	HP	123	107	94	0.134	0.116	0.102
	DV	144	135	131	0.157	0.147	0.142
А	DP	190	176	164	0.219	0.203	0.189
	HP	164	139	115	0.189	0.160	0.133
	DV	166	141	131	0.192	0.163	0.151
С	DP	103	93	84	0.372	0.336	0.303
	HP	97	76	56	0.350	0.274	0.202
	DV	99	94	77	0.357	0.303	.0278
FA	DP	197	174	165	0.212	0.137	0.187
	HP	159	130	110	0.173	0.147	0.125
	DV	162	140	132	0.184	0.159	0.150
Т4	DP	194	172	169	0.215	0.201	0.190
	HP	149	127	106	0.174	0.149	0.124
	DV	161	140	132	0.188	0.164	0.154
ūγ	DP	177	165	156	0.224	0.308	0.196
	HP	144	121	100	0.180	0.152	0.126
	DV	157	137	128	0.199	0.172	0.161
Т2	DP	165	164	145	0.232	0.217	0.204
	HP	138	116	94	0.194	0.163	0.132
	DV	148	129	120	0.208	0.182	0.169
Tl	DP	151	139	130	0.256	0.236	0.221
	HP	131	109	87	0.222	0.183	0.148
	DV	136	118	110	0.231	0.200	0.187
FAC	DP	135	123	114	0.297	0.270	0.250
	HP	121	97	77	0.26€	0.213	0.169
	DV	122	104	96	0.269	0.228	0.211

^{*}See page 105 for abbreviation key.



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DRIVER	
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FERICO	1:195	PE41JE	1 < 1 P ,	PERICU	1 125	24 K L 00	181	PER100	1H10S	PERIUD	18125
0.1	^	1.4			3.4 3.5	12.1	3.41	16.1	1540	20.1	3.40
9.5		. 4	5.5	3.	1282	10.2	3.91	16.2	1528	20.5	300
5.0	6	4.3	25	· .	3141	12.3	40.6	16.4	1631	20.3	101
7.0	=	4.4	63	3.4	10401	12.4	075	10.4	1723	4.07	2.32
٦.٠	-	2.4	5.5	α υ•	15 KR	12.5	3.7.5	16.5	1651	20.5	274
9.0	•	5.4	10	x	+40.	12.5	255	16.6	7211	20.6	258
0.7	٥	1.4	7.4	9.7	2346	12.7	43,	10.7	2141	20.7	234
٥.0	0	4.3	16	~ 4	T .	12.4	40.5	16.9	2320	20.₽	243
9.0	0	7	0.0	c • a	5757	* * 7	1.5.1	10.3	2362	20.9	546
٦•١	7.76	5.)	8	١.,	12.54	13.0	372	0.11	2114	21.0	7)4
1.1	124	5.1	127		12.46	13.1	5:5	17.	17.15	21.1	.6.7
1.6	7.01	* 7	123		1132	13.2	4.0.5	17.2	1735	21.2	2.15
1.1	145	5.5	1 54	` • ′	1929	13.3	375	17.3	51.17	21.4	2+1
1.4	174	٠,	136	£ * 7	3.2 4	13.4	113	17,4	50b7	51.5	54.7
۱ • ۲	113	5.0	122	9.5	7 34	13.0	111	17.0	1 (+ 5 ?	21.5	455
1.¢	17)	5.5	216	9.6	7.5.7	13.6	15.3	17.6	2757	21.6	3.0
1 • 7	701	2.4	227	4.	6.65	13.7	344	17.7	2562	21.7	3.34
J . F	1.5	5 • 6	1.60	α • c	615	13.4	2.**	17.8	2543	21.8	3)
] • c	6.4	5.1	215		1,74,	1 3.	151	17.	6441	4.12	16
7.6	46	0.9	+1+	10.0	2.2	14.0	اعر	1.8.	1176	22.0	200
2.1	5.5	6.1	561	10.1	47.5	14.1	17 +	1 H . 1	. + 02	22.1	4 3 2
2.5	1.0	5	104	10.7	172	14.2	30.1	18.2	1111	22.2	310
< · 3	5.5	6.3	7 5 3	10.3	341	14.1	385	18.3	1828	22.3	31
5.7	(, ;	4.0	173	10.4	9.7	7.7	5.4.5	ъ•н1	1,521	22.4	315
2 • 5	X.	6.0	51+	10.5	29.4	14.5	348	1 θ.	1201	55.77	52
2.€	1 1	4.6	1357	10.6	778	14.6	2:03	18.6	127,	22.6	33,
2.7	6.9	6.7	1414	10.7	5 + 2	14.7	4.1.5	18.7	2011	22.7	7
۶.۵	40	E • 0	11.24	d	7.7	14.	1.45	H * 7	10%	25.€	350
2.3	6.9	6.9	1715	30.0	402	14.3	5 13	F . 4.	1035	22.3	117
3 • €	54.	7 • 1)	1691	2.1	1 0 7	15.1	5.0 A	10.0	2012	23.7	24
٠.	5.4	7.1	5 2 5 13	11.1	1:7	15.1	701	1 1	7 4 1	23.1	354
3.2	15	7.7	2413	11.	057	15.2	1.07	14.2	5.49	23.2	~ ~
3 - 3	6.5	7.3	2653	11.	C+ 2	15.3	8.7	1.4.4	627	23.3	3.8
3.4	44	7.4	2793	11.44	529	15.4	334	111,44	5.17	23.4	
3.5	4.6	7.5	2663	11.5	1.70	15.	7 × ×	13.5	f 1 +	23.5	5.3
3.6	50	7.6	3414	11.0	7.54	13.0	1034	13.0	1 5 5	23.6	3.53
3.7	4.5	7.7	1373	11.7	4,1 2	15.7	1377	1 ,. 7	3.15	23.7	3.21
g. 6	4 4	7 - 3	3475	, 11.	0 - 2		1144	8.5	107	23.8	334
3.5	77	6.7	3455	11.5	281	15.4	1168	19.	980	6.87	33
J • 4	1,	9•€	1107	12.€	7.61	16.0	10 +4	20.0	241	24.0	5.8

APPENDIX B

The second of the ACCOMMAND STATES SECTION OF THE S

SERIOD TAIPS		20.2 35P																																						
RIO, PER		2007																																						
PERIOD TR		16.2																																						
, i	200	4.3	450	695	41.0	200	4.45	5,10	515	477	01.5	205	4.55	7, 7.5	5.50	415	3.745	3.1.1	1.13	217	F 52.5	4.11	463	6.2.3	4.30	68.4	622	4.91	0.80	041	370	11.24	1)84	4011	1049	1470	1454	1540	1563	
1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	1.71	12.2	12.3	12.4	12.5	0.54	12.7	12.8	15.	13.	13.1	13.2	13.3	7. 1	(3.5		1.5.7	13.4	13.4		14.1	14.0	14.	14.4	1.5.	14.0	14.7	14.0	14.	15.	15.1	15.2	15.3	9.6.4	15.5	15.6	1 , , 1	15.2	15.4	
2111		4.1.11	4004	fons	524.71	33,14	7 11 7	1673	2333	1 201	1517	1.5.4	1203	5. 1.1	0 7 6	23.5	75.3	1.20	- 15	40.5	7:5:5	475	4.27	302	280	4 24	5 3.5	516	546	, 1 ,	111	14.	247	597	220	9.65	747	314	314	
611-111.			e * 4			٠.	,	a •	7.	5.0	1.1	•	F * C		£ .	. • (.	6.	2.	7.7	10.	1.7	10.	10.3	10.4	3 * ()	10.	~ · -1	10.4	7.07	11.5	1.1	111.3	11.3	11.4	11.5	11.0	11.7	11.0	11.	
(d) 21	14	100	46	6.9	÷	Ç	* * 4	- 61	201	7	1.4	× 4	163	16.0	154	291	1 6 2	108	67.5	1 2 1	750	5.83	1922	1103	1132	1925	1431	5175	2535	1.967	311)	1000	3534	3701	1512	1665	6 3 9 3	4513	447>	
PERIOD	7.5	2.4	4.3	J	4.5	4.6	1.4	H * 7	7 • 4	5.0	7.0	2.6	5 • 5	4.4	ć• ر	0.0	2	20.0	6. • 5	0.0	6.1	6.0	6.0	4.0	5.0	6.6	6.7	8.0	6.9	7.0	7.1	1.2	7.3	7.6	7.5	7.0	7.7	7.3	7. 3	
1 k 1 k 5	-	7	7	÷	0	0	ş	=	ŗ	101	51.5	571	243	172	143	135	131	120	171	~ T	117	46	11	42	49	1,1	74	р.3	7+	77	7.5	19	5.5	5.3	1+1	.,	5.4	15	2.4	
PERICE	0.1	0 • 2	0.3	0.4	9.0	9.0	0.7	d • U	5.0	J• C	::	1.2	-	1.4	4)	1.6	1.7	a.	1.9	2.0	2.1	2 • 2	2 • 3	2 • 4	7.5	2.4	2 • 7	2 · P	5 • 2	3•C	3.1	3.2	3.3	3.4	3.5	3.6	5.7	3.8	3.3	

APPENDIX B

TRIADS-PUREUF 1964 TRIP ACCUMULATIONS IN THE SYSTEM

--- MEDE DRIVER

*ARLE WORK

ERICO	TRIPS	PER100	TRIPS	PERICE	1×1PS	PERT-10	TALPS	PERIOD	TR195	PERIOD	IKIPS
0.1	0	;	6.2	8	4030	12.1	1364	16.1	2333	20.1	4
0.2	0	4.2	5.5	a	3874	12.2	1368	16.2	2282	20.2	3
0.3	0	4.3	5.5	80	37 +8	12.3	1425	16.3	1409	20.3	*
0.4	Э	4.4	7. 9.	B. 4	3675	12.4	1452	15.4	2510	50.4	ķ
0.5	0	4.5	6.1	8.5	3161	12.5	1325	16.5	230H	20.5	*
9.0	С	4.6	or or	9.6	3282	12.€	1507	16.6	3071	20.6	-
0.7	0	4.7	7 %	6.7	2430	12.7	1464	15.7	2916	20.7	*
9.0	0	4.8	1.6	e. e.	2679	12.8	1538	16.8	3226	20.9	3.0
6.0	0	6.4	100	5 • α	2437	12.	1554	16.9	3258	20.9	3
1.c	260	5.0	9.0	0.6	1673	13.	1332	17.0	29.31	21.0	7
1:1	592	5.1	1.55	1.6	1755	13.1	1464	17.1	3770	21.1	3.
1.2	240	5.2	132	9.5	1562	13.2	1365	17.2	36 34	21.2	36
	218	5.4	7 7 7	6	1476	13.3	1342	17.3	3712	21.3	57
J•4	203	2.4	149	4.6	1335	13.4	1325	17.4	36 JH	21.4	37
5.1	134	5.5	133	ŭ. 6	1119	13.5	1108	17.5	3243	21.5	3.2
1.6	142	5.6	731	A. D.	1294	13.6	1218	17.6	3570	21.6	37
1.1	121	7.5.7	247	6.7	1114	13.7	1151	17.7	3334	21.7	3.6
٩.	115	5.8	310	9.8	1075	15.8	1133	17.8	3250	21.9	36
1.5	117	5.9	343	5.7	1044	13.3	1090	17.3	3160	21.9	37
2 · C	7.8	0.0	343	10°C	A15	14.0	8.74	18.0	2480	32.0	3.00
2.1	34	1.0	601	10.1	12 B	14.1	1062	14.1	2535	75.1	3.0
2.5	8.2	6.2	651	10.2	080	14.	666	16.2	2330	22.2	36
2.3	7.3	6.3	7.93	10.3	450	14.3	1902	18.3	2194	22.3	36
2.4	7.5	4.9	H 72	10.4	8.31	14.4	1001	1 H - 4	2081	55.4	36
5.5	5.8	6.9	9:01	10.5	711	14.5	863	14.5	1516	55.5	7
5∙€	7.5	9.9	1440	10.0	E06	14.0	10+1	18.6	1523	22.0	3(
2.7	6.9	2.9	1507	10.7	758	14.7	1060	18.7	1304	22.7	3.6
5.₽	7.2	6.4	1736	10.9	769	14.3	1114	18.8	1274	22.8	36
2+3	7.1	6.9	1907	10.5	764	14.9	1134	18.9	121+	55.9	35
3.c	6.5	7.0	1833	11.0	665	15.0	1014	19.0	847	23.0	19
3.1	19	7.1	2534	11.1	34	15.1	1375	19.1	798	23.1	20
3 • 5	54	7.3	2671	11.2	811	15.2	1361	19.2	780	23.2	37
3,3	4.7	7.3	2365	11.3	826	15.3	1451	19.3	758	24.3	40
3.4	1.5	7.4	3132	11.4	н33	15.4	7851	17.4	727	73.4	6
3.5	74	7.5	3011	11.5	133	15.5	1338	19.5	518	23.5	3.2
3.6	54	7.0	3858	11.€	974	15.6	1784	19.6	565	23.6	4)
3.7	50	7.7	385C	11.7	763	15.7	1754	19.7	504	23.7	36
J. F.	3.7	7.8	3.374	н•11	1047	15.8	1840	19.8	481	23.8	3.7
3.9	20	6.7	3375	11.9	1073	15.9	1869	19.9	4.62	23.9	3,

SUM FE 24 HOUR RRIPS IN THE CATEGURY 22140
MINNPU VALUE
-14
MAXIMUS VALUE
CHECKSUM ERKER WORK HWYPER

INTAUS-PURIUE 1964 TALP ACCUMULATIONS IN FEE SYSTEM

--- MODE HWYPER

TABLE WORK

PERILO	TRIPS	2FR100	TRIPS	DEPTED	12105	PERIL	17.1.35	PER100	T31P5	PERIOD	TRIPS
0.1	10	4.1	9	α.	5060	12.1	1547	16.1	2914	20.1	5
0.2	c	4.2	44	8.2	α	12.2	1556	16.2	2863	20.2	3
0	0	4.	7.3	m. m	4727	12.3	1627	16.3	3032	20.3	483
4.0	0	4.4	17	A . 4	4566	12.4	1665	16.4	3166	50.4	4
0.5	0	4.5	60	A. 5	3894	12.5	1528	16.5	2920	20.5	3.6
9.0	2	4.6	101	η. • κ	3534	12.6	1741	16.6	3933	20.€	4
0.7	0	4.7	36	7.0	3529	12.7	1695	14.7	3814	20.7	3
H • 0	0	9.	114	о • в	3202	12.8	1780	16.8	4150	8.02	4
6.0	9	6.7	118	<i>у</i> • а	28.45	12.9	179₺	16.9	4210	50.9	4
1.0	343	0°¢	103	3.€	1346	13.0	1539	17.0	3303	21.0	~
1.1	356	5.1	164	5.1	1+07	13.1	1681	17.1	4843	21.1	4
1 . 5	111	5.2	141	9.1	1800	13.2	1567	11.2	4762	21.2	4
1:3	.'83	5.3	184	5	1690	13.3	1539	17.3	4118	21.3	4
1.4	264	5.4	194	5.6	1596	13.4	1518	11.4	4763	21.4	4
91	16.3	5.5	175	6.5	1273	13.5	1266	17.5	4199	21.5	ż
1.¢	182	5.0	103	9.6	1 147	13.6	1391	17.6	4582	21.6	4
1.7	155	5.1	326	6.3	1265	13.7	1314	17.7	477H	71.7	4
1.5	f 51.	5.8	774	9.6	1215	13.8	1246	17.8	1915	21.B	4
1.5	143	5.4	494	5.6	1179	13.9	1236	17.9	4050	21.9	4
2 · C	105	6.3	467	J*U1	414	14.0	532	18.0	3160	22.0	3
2.1	1.50	6.1	P 15	10.1	1033	14.1	1214	18.1	3272	72.1	4
2.2	111	6.2	8 + 5	10.2	971	14.2	1142	18.2	7167	12.2	4
2.3	196	0.1	1043	10.3	936	14.3	1147	18.3	2121	22.3	4
. 2.4	46	6.4	1111	10.4	911	14.4	1147	18.4	5,849	55.4	4
5.7	4.3	6.0	1223	10.5	174	14.5	266	18.5	1869	22.5	ň
2.€	11	4.0	1974	10.€	P.84	14.6	1272	19.6	1878	22.6	7
2.1	6.6	6.7	2069	10.7	326	14.7	1236	18.7	1595	22.7	4
2 · P	6.8	6. A	2361	10.4	R32	14.8	1309	18.8	1545	22 · B	4
5.5	200	÷.	2504	10.9	826	14.9	1335	18.9	1468	55.9	4
3∙€	7.5	0.7	2485	11.0	718	15.0	1202	1 1.0	100%	23.0	ě
3.1	62	7.1	3380	11.1	016	15.1	1656	1 9.1	1050	23.1	4
3.2	64	7.2	3435	11.2	184	15.2	1645	19.2	928	23.2	4
3 • 2	2.5	7.3	3873	11.3	901	15.3	1776	19.3	006	23.3	4
3.4	56	7.4	4081	11.4	305	15.4	1926	19.4	564	23.4	7
3.5	95	7.5	3890	11.5	806	15.5	1667	19.5	6 1R	23.5	4
3.6	69	7.6	1 565	11.6	1073	15.6	2257	19.6	580	23.0	Š
3.7	Y C	7.7	4+31	11.7	1959	15.7	2219	10.7	608	23.7	4
3.6	90	7.8	5071	11.8	1157	15.8	2322	19.8	586	23 • B	3
3.5	59	4.7	5054	11.5	1197	15.9	2343	19.9	581	23.9	4
4	(7	a	4600	12.0	1107	16.0	2145	0 176	7.44	24.0	7

SLW CF 24 FCLR FAIPS IN THE CATEGORY 27135 FININGW ALLE PARTHEW VALUE CHECKSUM FARCH WUNK ALLPER

LATADS-PURICS 1964 TRIP ACCUBULATIONS IN THE SYSTEM
TIPLE PRISHOP --- MODE DAILNER

		- I		1,198	01.100	14375	PEN100	14105	PERIOD	14125
Ð	4.1	2	α.	12	12.1	5.30	16.1	602	20.1	H 25
0	4.2	2	н.2	3.6	12.2	206	16.2	7+5	20.2	111
0	4.3	2	α	61	12.3	514	16.3	524	20.3	762
0	5.4	2	7. ×	44	12.4	6.05	16.4	6.29	20.4	748
0	4.5	-	υ° π	8.7	12.5	4.2.1	16.5	615	20.5	61H
0	9.4	3	H.6	113	12.6	446	16.6	673	20.6	662
0	4.7	3	ч. 7	114	12.7	454	16.7	069	20.7	634
0	4.4	4	g.	129	12.	477	16.8	707	20.8	629
0	6.4	J	τ·	136	12.9	474	16.9	733	70.3	642
12	٥.٠	~	0.6	1,1	. 3.	6.20	17.0	4.4.5	21.0	222
11	5.1	5	'.1	1.48	13.1	526	17.1	754	21.1	548
	5.2	٠.	1.5	2115	13.2	52.5	17.2	743	21.2	515
	5.3	5	5.5	177	14.3	4 + H	17.3	774	21.3	506
	5.4	Ş	4.6	235	13.4	513	17.4	7.03	21.4	506
٦	5.5	v.	5.6	227	14.5	462	17.5	654	21.5	615
-	3.6	9	9.6	2 414	14.6	525	17.0	1 34	21.6	405
_	7.4	9	1.7	2 7:3	1 3 . 7	513	17.7	7 8 1	21.7	367
7	5.4	4	~ · ·	111	13.0	573	17.8	784	21.8	103
5	2.0	7	3.3	3.4	13.	c:	17.9	406	21.,	305
0	0.9	٣	0.01	41.2	14.)	4411	18.0	6.4%	22.0	283
7	4.0	1.5	1.0.1	6.6.45	14.1	2 30	16.1	0 3.7	22.1	116
0	6.2	۲,	10.2	Ç q	14.2	5,15	16.2	80×	75.77	243
0	6.3	14	10.3	517	14.5	179	18.3	P. 3.1	22.3	231
2	4.0	۲2	10.4	513	14.4	53H	19.4	421	24.4	223
Э	6.5	14	10.5	465	14.5	460	16.5	1.07	22.5	151
0	6.6	1.5	10.6	5.71	14.	124	18.6	13 to D	22.6	141
· ·	1.9	57	10.7	595	14.7	4.48	18.7	#2"	22.1	122
c	6.8	1.3	10.A	661	14.4	504	18.8	952	27.P	011
G	6.0	6.1	6.01	109	14.9	010	6.41	X t	24.3	1:04
О	7.0	16	ा।•ः	517	15.0	448	0.01	73.5	23.0	6.5
0	7.1	23	11.1	629	15.1	7.30	1.9.1	404	23.1	09
2	7.2	4.7	11.2	586	15.2	+0+	19.2	710	23.2	*
0	1.3	33	11.3	616	15.5	, 11	15.3	884	23.3	4.6
٥	1.4	35	11.4	9.30	15.4	5.2.5	1.4.4	, ę. a	73.4	4.9
0	7.5	3-3	11.5	511	15.5	494	19.5	784	23.5	28
7	7.6	6.5	11.6	603	15.4	6.50	9.61	3.6	73.6	5.3
-	1.1	49	11.7	578	15.7	533	19.7	8.6.1	23.7	2.6
-	7.8	29	11. Р	576	15. H	558	1.5.8	B7.4	23.R	56
3.9	7.9	69	11.9	542	15.79	095	19.9	6.9.5	63.9	2 3

SUM CE 24 FOLK TKIPS IN THE CATEGORY 7345 MINDHOW VALUE PAKTHUW VALUE CHECKSUM CPROK HUSHUP FWYPER

INTADS-PUPTUE 1964 TRIP ACCUMULATIONS IN THE SYSTEM

TAPLE HBSHOP --- MODE HWYPER

INIPS	1477	1390	1361	1347	1118	1208	1159	1152	1175	154	482	406	843	514	705	284	619	6.0.3	2+5	449	445	385	363	352	230	-16	1.85	171	161	3.6	a p	12	7.1	5 +	7 5	55	0.4	47	43	3.3
PERIOD	20.1	20.2	20.3	20.4	20.5	20.6	20.7	20 • H	20.9	21.0	21.1	21.2	21.3	21.4	21.5	21.6	21.7	21.8	21.9	22.0	22.1	22.2	22.3	55.4	22.5	24.6	22.7	22.8	57.0	23.0	23.1	23.2	23.3	23.4	23.5	23.6	23.7	23.8	23.)	24.0
18 LP 5	120	216	988	368	9.3.5	105H	2201	1107	1137	1992	1143	1117	1141	1175	1013	1202	1185	119,	1214	1051	1226	1224	125H	1621	1101	1345	1243	1324	1341	116.1	1447	14,4	1482	14.34	1326	1514	1473	1516	1518	1376
PERIUD	10.1	16.2	16.3	16.4	16.5	16.0	16.7	14. "	16.4	17.	17.1	17.2	17.3	11.4	17.5	17.6	17.7	17.4	17.1	18.0	18.1	16.2	18.3	18.4	18.5	18.0	1 4 . 7	I. d.	1:-	19.0	1.0.1	19.2	19.3	1 4.4	19.5	19.6	13.7	19.8	6.61	20.0
TRIPS	7.38	/11	720	722	675	6.3.2	259	673	5+5	2+1	147	643	469	710	643	7.45	717	7+0	729	F()4	£4.	725	743	753	249	1 + 1	10 m	710	124	537	176	747	754	761	076	808	142	∺31	434	911
001898	12.1	12.2	12.3	12.4	12.5	12.6	13.7	0./	17.3	13.0	13.1	14.2	13.3	13.4		14.6	1 . ,	. • . 1	15.3	14.',	1.4.1	14.2	14.3	14.6	14.5	14.0	1 /	~ * 5.1	14.4	[5.7	15.1	15.0	15.4	15.4	15.5	15.6	10.7	15.8	15.4	16.0
TRIPS	116	116	122	120	109	145	051	163	176	159	251	616	2 11.	313	30.2	905	4016	441	144	57.5	ちたち	659	011	649	6.3.1	764	126	70.7	406	* H 0	4 7	1+5	717	51.1	604	803	715	7.4	742	2640
PERICE	9.1	2.0	8.0	9 ° 6	8.5	9.0	В.7	H .	ar ar	9.6	9.1	2.6	.4.3	7.6	3.6	9.¢	1.7	9°6	5.6	10.0	10.1	10.2	10.3	10.4	10.5	10.6	10.7	10.8	10.3	11.0	11:1	11.2	11.3	11.4	11.5	11.6	11.7	11. 1	11.0	12.0
TR1P5	P.1	2	2	2	~	*	~	0	¢	5	7	1	7	τ	7	r	σ	Ŧ	7	α	17	14	5.7	16	5.1	17	11	57	2.2	1 +	C.E	36	41	4.5	(5	я 3	8.2	86	C.E	3.5
PERIOC	4.1	2 * 5	4.3	4.4	4.5	4.6	1.4	x r	6.4	6.0	5.1	5.2	6.4	5.4	5.5	5.6	7.4	5.8	F.4	0.9	0.1	2.9	6.3	5.44	6.5	9.9	6.7	6 • B	4.4	7.0	7.1	7.2	7.3	7.4	7.5	7.6	7.7	7.8	7.4	8 °O
TRIPS	0	С	0	0	Э	0	C.	С	9	91	16	14	10	10	20	~	~	4	2	7	-	1		7	0	0	0	c	כ	c	÷	C	0	6	0	-	-	-	-	-
FERICE	0.1	3.5	0	9.6	9.5	0.e	7.0	0.6	5.0	١٠٠	1.1	1.2	1.3	1.4	· ·	1.6	1.7	1.8	1.9	2.5	7.1	3.5	2 - 3	2.4	6.5	7.6	2.1	a . S	2.3	3.0	3.1	3.2	5.3	3.4	3.5	3.6	3.7	3.4	3.9	7.4

INTADS-PURTUE 1964 TRIP ACCUMULATIONS IN THE SYSTEM

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OR I
MODE
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100						1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	-	1	; ; 1		
-	0	**	40	1 • N	42 8 7	12.1	1578	16.1	3941	20.1	2411
2.0	=	7.4	6-0	H.2	4142	12.2	1534	16.2	17.65	20.7	2240
2	0	4.3	19	80	4004	12.3	1557	16.3	1126	20.3	2298
. 4	0	4.4	7.1	A . 4	3405	17.4	1570	16.4	1208	20.4	2167
u)	2	4.5	6]	8.5	3425	17.5	1358	10.5	2419	20.5	1752
• ¢	0	4.6	9.1	3.6	1651	12.6	1573	10.6	3732	20.6	1897
٠.	C	1.4	9.6	н. 7	5353	12.7	1510	10.7	1991	20.7	1743
æ	0	H. 4	102	er er	5133	12.8	1562	16.8	3417	20.A	1748
5.	⊃	6.4	104	E . A	2873	12.	1575	16.9	3485	6.07	1759
٥.	3.8.3	5.0	65	3.0	205R	13.0	1334	17.0	3529	21.0	1442
-:	33.8	1.0	142	7	5577	1 3.1	1626	17.1	86.55	21.1	1643
	344	2.5	133	6.3	2040	13.2	1538	11.2	4428	21.2	1637
۳)	171	5.3	150	E · f	1956	13.3	9551	11.3	4547	21.3	1602
٠,	307	5.4	154	3.4	1878	11.4	150,5	17.4	4584	21.4	1615
5	715	5.6	139	6.5	1532	13.5	1344	17.5	4.05.9	21.5	1327
÷.	213	5.6	047	9.6	1673	13.6	1516	11.6	1695	71.6	1492
٠.	183	5.7	253	3.7	1551	13.7	1465	11.7	4474	21.7	1405
3.	190	5.8	15.3	α. 6	1528	13.⊬	1467	11.8	4456	21.8	1428
5	173	5.3	353	9.9	5747	13.0	1423	17.9	7555	21.9	1444
٠.	122	0.9	3.52	10.€	1156	14.0	1136	0.5.1	3674	22.0	1181
-	149	0.1	635	10.1	1490	14.1	1471	1×°	4126	22.1	1361
- 5	129	6.2	678	10.2	1396	14.2	1334	14.2	3841	25.2	1770
۲.	171	6.3	823	10.3	1 179	14.3	1411	14.3	3953	22.3	1274
4	120	4.0	168	10.4	1364	14.4	1711	14.5	1150	72.4	1266
u ı	14	6.0	921	10.5	1186	14.5	1254	18.5	7498	22.5	1012
٠,	121	9•0	1503	10.6	1383	14.5	1539	13.6	1131	22.6	1129
. 7	112	6.7	1941	10.7	1322	14.7	1531	18.7	3070	22.7	1035
α.	113	8.9	1794	10.8	1349	14.8	1636	18.8	5131	22.8	1063
5.	110	6.9	1833	10.9	6581	14.4	1672	18.9	10+3	22.4	1051
ပ•	98	0.7	1874	11.0	1157	15.0	1531	10.0	(-542-)	23.0	8 12
-	3.6	7.1	2600	11:1	1414	15.1	1.17.0	13.1	2913	23.1	5.63
2.	7.8	1.2	2690	11.2	1349	15.2	1960	1.3.2	2748	23.2	4)4
~1	£9	7.3	5967	11.3	1356	15.3	2106	1 3.3	2761	23.1	84.8
4.	19	7.4	1123	11.4	1373	1,5.4	2174	1 4	2774	23.4	01H
٠.	53	4.5	5667	11.5	1209	15.5	1117	19.5	2344	23.5	7117
٠,	99	7.6	3937	11.6	1439	15.6	2507	9.4.1	2683	23.6	A24
٠.	65	1.1	3919	11.7	1399	15.7	1447	11.7	7556	73.7	757
a)	96	7.B	4106	11.8	1482	15.8	5252	14.8	4885	23.B	172
5.	2.5	6.7	4130	11.9	1501	16.9	2604	0.01	26.74	0.10	176
									0 1 5 3		-

| TADL-PORTURE FASA FATO ACCOUNTING IN THE SYSTEM | TABLE PRINT | --- | PURE PRINTER

DD TRIPS	7	7	7		,	(4)					. 4			10					9 24RN		,					_									_		_	~		
PEKIOD	20.1	20.	20.	20.	20.5	20.0	20.1	20.	20.0	21.0	21.1	21.6	21.	21.	21.5	21.	21.	21.	21.4	75.1	75.1	22.0	22.	75.0	22.	22.1	22.	22.	22.	23.0	23.1	2 3 . 4	23.	23.4	23.	23.6	23.	23.6	23.	
TRIPS	4725	76.55	4772	4862	43.34	5587	5 37 1	279.	5495	5213	6540	6453	6636	667	5914	6.784	6502	6443	1440	5316	2109	5681	5644	2045	4470	5004	4634	4839	4803	3910	4.8.5.4	46.13	1264	4755	4087	4700	4534	4640	4622	1 -
PERIUU	16.1	16.2	16.3	16.6	16.5	16.6	11.7	16.8	16.0	17.0	17.1	17.2	17.3	17.4	17.5	17.6	17.7	17.4	17. 4	0.41	18.1	14.2	1.6.3	18.4	18.5	10.6	18.7	18.x	18.	10.0	10.1	14.2	1.1.3	14.4	10.5	19.6	11.7	13.4	14.3	4 1
18 I	7261	21.18	2227	1 4 2 2	1.50	27 Ac	2202	01.7	2344	1333	2 141	2230	2237	2239	1334	2176	2398	2038	2027	1053	6507	1972	5649	2041	1815	1.04.2	6213	6982	2475	2175	2052	2+36	3263	1100	3108	3.385	3432	4115	4138	
0.11 - 22	12.1	12.2	12.3	12.4	1 ? • 5	12.6	14.7	16.8	17.9	13.0	13.1	13.2	13.3	13.4	13.5	13.6	13.7	3 · · ·	14.9	14.	14.1	14.	14.	14.4	14.5	14.	14.	14.9	14.	13.1	15.1	15.2	15, 4	15.4	15.5	15.0	15.7	H.C.1	15.9	
18105	6139	6.003	3346	5721	1.000	5385	4 × 3 4	1677	41.34	24.62	1125	. 841	2772	2616	2079	2744	2120	2342	1995	1558	1978	1870	1851	1 44 1	1614	1457	1762	1869	1912	1554	1915	1832	1268	1.40 /	1012	1.98.5	1330	2042	2076	
9 'R1LF	в.1	8.2	C) a	7.4	9.5	9. H	r . 7	X * X	6.8	J• 6	~	3.5	7.3	7.5	E. G.	3. t	7.0	1.9	6.6	10.0	10.1	10.2	10.3	10.4	10.5	10.6	10.7	10.P	10.9	11.0	11.1	11.2	11.3	11.4	11.5	11.6	11.7	11.8	11.9	
1416.	O.R.	7.5	8.7	3.1	٥٩	113	107	124	126	112	174	173	186	195	173	115	336	437	431	707	A71	343	1156	1256	1621	2055	1412	7444	25R6	9557	3523	3653	4068	4 3 0 7	4126	5503	5533	5879	2940	
	4.1	7.5	4.5	4. * 4,	4.5	4.0	1.4	6.4	4.4	0.€	5.1	5.5	5.3	5.4	5.5	٠, د	5.7	5 • B	5.0	0.4	¢.1	6.2	6.3	4.9	6.5	4.6	2.0	6.8	6.9	7.0	7.1	7.2	7.3	1.4	7.5	7.6	1.1	7.8	7.9	
Talps	c	-	2	-	=	0	C	-	>	586	270	504	794	455	3.04	117	271	1,5,	1,62	180	417	184	185	111	1 16	171	152	151	143	116	126	106	36	3.5	7.7	85	7.5	12	74	
PERICE	0.1	0.0	0.0	4.0	0.5	9.¢	0.7	9.0	5.0	J.C	1.1	1.2	1.3	7.1	41.	1.6	١.٠	7. T	ਾ•1	2 • U	2.1	2.5	2.3	5.4	2.5	2.6	2 . 7	2.8	5.5	3.0	3.1	3.2	3.3	3.4	3.4	3.6	3.7	α. Έ	3.5	,



1:TAUS-PTWOVE 1964 TKTP ACCUPULATIONS IN THE SYSTEM TANGE TOTAL --- FINE RAINE

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559 OF 24 ECONTAINS IN THE CATECOUN -8815 MANNEW VIOLE -42 ENTINES VALUE CHECKLOUN FORC TITAL 411PER

INTADS-PUREUE 1964 TRIP ACCUMULATIONS IN THE SYSTEM

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2007
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TOTAL
4 J J J V

PERICE	TRIPS	0E # 100	TP 1 P S	PFRICE	18195	PERICE	TRIPS	PERIOD	TRIPS	PERIOD	IRIPS
0.1	2	4.4		x	7101	12.1	2125	16.1	6383	20.1	5410
2.0	Ú	4.2	8.5	æ.	6916	12.2	4144	16.2	6201	20.2	50.63
0.3	0	4.3	061	B. 3	5878	12.3	40.4	16.3	6421	20.3	5050
0.4	Э	4.4	106	A . A	6705	12.4	4264	10.4	0.540	20.4	4955
0.5	-	4.5	ř	×.5	5,859	12.5	3787	16.5	5.327	20.5	4637
9.0	Ξ	9.5	132	8.6	6226	12.6	4336	16.6	7381	20.6	4445
0.7	5	4.7	127	4.7	5699	12.7	4186	16.47	7125	20.7	4 [O B
9.0	0	A . A	148	Œ.	5307	12 • a	4 1 4 7	16.8	7631	20.₽	4671
5.0	5	6.4	153	υ· κ	4876	12.3	4455	6.01	1742	5.07	6434
J•1	164	5.0	141	J • 6	5552	13.0	3803	17.0	6.300	21.0	3300
1.1	4.74		203	9.1	2P34	13.1	432B	17.1	8508	21.1	3726
1.2	502	5.2	207	9.2	3574	13.2	65(5	17.2	6368	21.2	3540
1.3	54.1	5.3	223	9.3	3426	13.3	4)64	17.3	4533	21.3	34 45
1.4	225	5.4	2 33	4.4	3311	13.4	1505	17.4	H552	21.4	3.7.7.5
1.5	11/22	3.5	217	5.5	2715	13.5	3504	17.5	7558	21.5	26.36
1.6	218	£.	365	9.6	3.038	13.6	3337	17.6	6.515	21.6	3143
1.1	3.T.s	100	387	1.7	1687	13.7	3738	17.7	H122	21.7	2+71
1.8	503	er (5)	Cf 5	9.6	29)3	13.A	3739	17.8	6906	21.8	3036
1.9	232	5.4	in the	6.6	6,87	13.7	3617	17.4	7349	21.3	3045
2 • C	213	0.0	546	10°C	2312	14.0	1617	16.0	6.561	22.0	54 43
2.1	643	6.1	754	10.1	2913	14.1	1584	18.1	7370	22.1	2761
2 • 2	220	4.2	1046	10.2	2320	14.2	3427	14.2	684.1	22.2	2550
2 - 3	217	6.3	1264	10.3	2843	14.3	3488	Ι α• 3	2524	22.3	2515
5.4	20%	4.0	1393	10.4	2942	14.4	3548	10.4	4514	22.4	24 12
2.5	160	0.5	1437	10.5	2358	14.5	3764	17.5	5375	52.5	20.00
2.6	1.34	9.9	2263	10.6	5967	14.	3740	18.6	6747	22.4	51.43
2.1	173	11	2375	10.7	2872	14.7	3510	14.7	1584	22.1	2074
2.P	170	7 . 3	2762	10.P	5314	14.3	3734	14.5	1, 4, 2 1	21.7	2042
5.5	167	f • 9	2871	10.3	50.12	· • • 1	3866	1 F • 3	1695	777	2016
3.6	130	7.0	2445	11.C	2626	15.0	3450	1.4.0	1694	23.0	1621
3.1	133	7.1	3883	11.1	4224	15.1	6055	10.1	5717	23.1	54.67
3 • 5	116	1.5	4047	11.2	31.05	15.7	4367	14.2	1447	23.2	1754
3 • 3	105	1.3	4532	11.3	3147	15.7	4748	10.3	5195	23.3	1768
3.4	102	7.4	4815	11.4	3254	15.4	4870	11.4	166 +	23.4	1717
3.0	1.3	7.5	4631	11.5	2 305	15.5	7155	19.5	5 £ £ 5	23.5	1425
3.6	34	9.1	4167	11.6	3489	15.6	5,10	19.6	5691	23.6	1291
3.7	8.5	1.1	6250	11.7	3406	15.7	5315	19.7	5486	73.7	1434
3.8	95	7.8	6636	11.8	3649	15.P	542H	19.8	2636	23.H	1516
3.9	85	7.9	6736	11.5	3792	15.9	5667	11.4	2911	23.9	1515
7.4	60	0°4	6304	12.0	3378	0.91	5122	50.0	4925	24.0	1218

TRIP CCUNTS FOR SAMPLE 1 BY MODE ORIVER

HBDTHR	0	0	0	0	7	7	01	7	0	2	5	4	3	2	91	32	28	91	9	00	œ	5	80	9
HBSREM	01	4	7	~	0	01	7	15	22	35	3.2	3.2	38	23	39	52	16	101	115	78	58	47	36	11
H8 P B M D	-	0	0	0	0	7	œ	16	22	33	22	31	28	54	56	22	18	31	18	7	10	7	3	-
HB-SCL	0	0	0	0	0	2	51	94	4	œ	16	61	~	78	2.7	10	71	ю	3	2	2	~	7	0
нв ѕнор	0	3	1	~	1	2	11	19	45	85	84	2.1	69	62	20	104	104	90	105	7.2	38	6	4	2
+Bhokk	00	œ	8	11	36	201	387	151	20	2.2	3.1	5.8	39	7.2	123	250	529	95	28	12	35	3.8	25	25
CTHER	1	0	0	0	1	6 0	11	3	0	5	2	ç	3	9	70	4 1	3.2	1.7	9	6	6	9	6	4
S	12	2	7	~	0	11	7	18	25	6\$	45	43	46	34	64	63	26	114	138	66	13	99	47	13
P B - M C		U	٠	۲,	0	~	σ	5.0	27	45	34	47	55	37	27	2.8	2.1	3.7	21	6	13	4	3	2
SCH001	0	0	0	0	9	5	65	20	5	•	18	54	2	2.1	3.8	14	15	10	٤	7	9	-	-	0
SHOP	0	0	-	2	2	2	1.2	53	5.8	128	110	8.5	8.6	66	13	129	125	113	137	36	65	13	5	2
HORK	10	ec	6	14	55	2.30	400	503	107	89	124	180	126	149	217	350	242	108	040	21	4.1	55	1	33
HCUR		2		4	5	9	7	8	6	01	1.1	1.2	13	14	15	91	1.7	18	61	20	2.1	22	23	54

TRIP CCUNTS FOR SAMPLE 1 BY MODE PASNGR

H801HR	0	0	0	0	0	4	2	~	0	0	7	2	0	0	2	3	3	7	٩	0	-	7	9	0
HBSREM	'n	3	5	7	~4	~	11	7	18	7	21	52	20	81	56	77	53	43	131	8.2	84	84	35	1.7
HBPBMO	0	0	0	0	0	-	-	7	12	10	6	17	9	9	6	15	7	12	11	4	٣	-	0	3
HB-SCL	0	0	0	0	0	3	44	51	3	٩	17	21	4	12	62	9	10	7	2	2	~	0	7	1
нв ѕнор	0	0	0	7	0	-	4	9	21	54	56	56	97	54	28	45	20	58	98	49	19	4	0	-
H8 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	2	~	7	2	13	99	118	2.7	7	3	4	10	4	18	42	7.8	89	20	12	11	9	9	6	7
C THER	0	0	0	0	0	4	5	1	0	0	-	2	0	0	2	4	3	~	4	0	1	~	4	0
S	5	~	0	-	-	3	12	œ	20	80	53	33	52	22	58	5.1	63	101	148	100	62	61	45	18
P 8 - N	O	O	0	0	O	~	2	œ	14	1.7	12	18	ا د	13	10	1.7	6	16	14	4	4	1	0	3
SCHOOL	0	0	0	0	0	. 3	80	53	4	4	20	28	2	14	67	3 0	12	01	4	3	4	~	2	7
SHOР	0	0	0	2	0	-	4	7	54	39	36	37	3.7	3.7	42	95	63	99	109	14	22	01	0	-
T OR K	3	-	2	2	15	69	129	30	13	S	19	38	01	52	63	93	93	52	16	14	ono.	7	01	oc
HCUR	1	2	e	4	2	9	7	6 0	5	10	11	12	13	14	15	16	17	1.8	61	50	21	22	23	54

TRIP CCUNTS FUR SAMPLE I BY MODE TRNSIE

HBOTHR	0	0	0	0	0	0	0	0	0	0	0	0	0	7	7	2	0	3	0	0	0	0	0
HBSREM	0	0	0	0	0	14	-	Þ	-	0	0	7	0	0	0	0	•	7	-	2	~	7	1
H8P8MD	0	0	0	0	0	0	0	2	7	3	0	4	4	3	7	3	2	0	0	0	0	0	0
HB-SCL	0	0	0	0	0	7	183	163	2	2	5	12	0	68	198	22	-	-	2	4	-	0	0
нвѕнор	0	0	0	0	0	0	0	2	3	5	4	3	7	2	-	2	2	7	-	3	0	0	0
T & S O R *	0	0	0	0	7	14	38	23	6	9	3	0	5	3	15	54	77	10	7	4	٦	1	2
CTHER	0	0	0	0	0	0	0	0	0	0	0	0	0	1	7	2	0	3	0	0	0	0	0
SRIE	0	0	0	0	0	7	7	0	1	0	0	1	0	0	0	0	٣	2	1	2	1	1	1
P8-F0	ر.	0	0	0	0	ပ	0	3	2		0	4	4	4	3	E.	2	O	0	С	Ö	0	С
SCHOOL	0	0	0	0	0	2	183	164	2	2	5	12	7	06	199	54	7	-	2	4	7	0	0
SHOP	0	0	0	0	0	0	0	2	٣	9	4	9	2	5	2	4	7	3	1	3	~	0	0
1 0 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	0	0	0	0	7	14	45	97	6	9	9	_	9	4	91	30	909	=	7	2	3	-	2
HCUR	-	2	3	7	5	9	7	œυ	6	01	11	12	13	14	15	91	11	18	19	20	21	2.2	23

AVERAGE TAIP LENGTHS FOR SAMPLE I BY MIDE DRIVER

H801HR	0000	6.0 14.3 12.1	9.2	12.0 5.7 10.4 9.4	13.9 10.2 8.8 12.9	10.7 11.8 11.6 7.7
HB SR EM	15.4	0.0 8.2 10.1	10.7	12.8 14.0 13.4	12.5	10.6 14.7 15.5 15.6
нврвмо	20.0	17.0	13.0 14.8 14.9	13.1 12.5 15.9	10.6	14.0 18.5 9.3 22.0
H8-SCL	••••	13.6 10.8	17.0	8.7 9.0 9.8 10.1	18.2	14.2 28.0 28.0 0.
HESHCP	0. 0.7 31.0	11.3 11.3 11.3	10.0	10.3 11.5 9.4	0.01 6.9 7.9	9.5 12.6 9.5 16.0
HBWORK	16.6 15.9 20.4 18.0	16.9	16.1 17.5 16.2	14.4 14.4 17.1 16.1	18.8 15.2 15.4	15.3 18.0 18.3 17.5
OTHER	19.0	11.55	9.2 9.2	12.c 5.7 9.3 10.3	14.5 10.0 11.9	10.2 11.6 12.4 7.2
SRIEM	15.8	0.01	12.0	11.7	13.1	11.1 14.4 16.0 15.2
P8-M0	20.0	15.9	13.1 13.6 14.5	12.2	13.7	14.5 15.5 9.3 17.5
SCHOOL		13.6 10.3	17.2	9.0 8.5 10.1 10.5	11.8	14.8 28.0 28.0 0.
SHOP	0.00.00.00.00.00.00.00.00.00.00.00.00.0	11.4	11.2	10.7	9.6	9.6 14.2 10.8 16.0
MORK	18.6 15.9 20.8 15.6	16.7	13.6	12.1 13.4 14.9 15.1	17.5	14.5 17.4 17.4 17.7
HOUR	7 3 5 7	. v o r a	010	12 13 14 15	17 19 20	21 22 23 24



AVERAGE TRIP LENGTHS FOR SAMPLE I BY MCDE PASNGR

HBSREM HBOTHR	00.00.00.00.00.00.00.00.00.00.00.00.00.	
18PBMD HBSF	0.000000000000000000000000000000000000	
HB-SCL	0.000000000000000000000000000000000000	15.0 13.0 0. 22.0 19.0
HESHCP	00000000000000000000000000000000000000	10.7 11.3 12.2 0.
HBWORK	2.5 2.5 2.5 2.5 2.5 2.5 2.5 2.5 2.5 2.5	15.3 10.0 13.7 14.1
OTHER		0. 36.6 6.0 26.7
SR-EM	0.000000000000000000000000000000000000	12.2 13.4 14.7 13.3
9 E E E E E E E E E E E E E E E E E E E	00000000000000000000000000000000000000	12.0 12.0 13.0 0.
SCHOOL	00000000000000000000000000000000000000	13.7 11.5 17.0 22.0 19.0
SHOP	00.00000000000000000000000000000000000	10.9 11.3 14.2 0.
MORK 	8.0 15.0 15.0 17.7 17.7 17.7 17.7 17.7 17.7 16.9 16.9	15.4 10.4 12.7 13.3
HOUR	126432	20 22 23 24

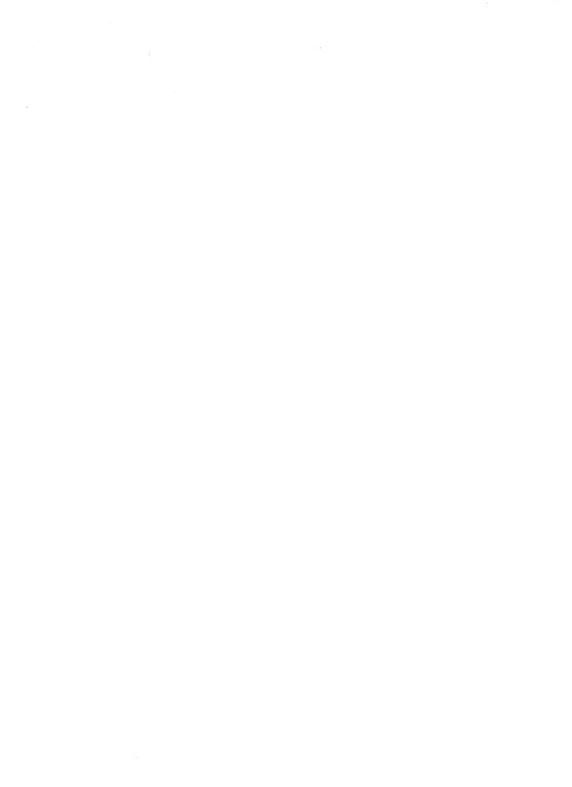


AVERAGE TRIP LENGTHS FOR SAMPLE 1 BY MCDE TRNSIT

HBOTHR	000000000000000000000000000000000000000	•
HBSREM	000000000000000000000000000000000000000	• o
H8P8M0	20000000000000000000000000000000000000	٠,
HB-SCL	00000000000000000000000000000000000000	•0
HESHCP	00000000000000000000000000000000000000	•
HBWDRK	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	13.5
ОТНЕК	00000000000000000000000000000000000000	•
SRIEM	000000000000000000000000000000000000000	•
P8-MD	00000000000000000000000000000000000000	•
SCHOOL	000000000000000000000000000000000000000	•
SHOP	00000000000000000000000000000000000000	•
* DRK	00000000000000000000000000000000000000	13.7
HCUR	20000000000000000000000000000000000000	47







```
APPENDIX C
               SMAP
SIBETC PEAKS
      INTEGER P1, P2, P3, MODE(15), PURPF(15), PURPP(15), PURPT(15), TIM
     les(15), rimea(15), count(241,2,3,10), record, cmd(6), purp(9)
      INTEGER IS, TA
      INTEGER HOUR(240)
      INTEGER SUM
      CIMENSION TABLE(10), HOW(3), HH(241), WORK(500)
      DIMENSION X(242), Y(242)
      EQUIVALENCE (X(1), HH(2))
      CIMENSION TITLE(4), T301(3)
      DATA TITLE/6HENO OF,6H PERIO,6HD - 0.,6H1 HOUR/,T301/6HTRIP 4,
     $6HCCUMUL,6HATIONS/
      DATA TABLE/6HWORK ,6HSHDP ,6HSCHOOL,6HPB/MO ,6HSR/EM ,6HOTHER ,
     $ 6HHBWORK, 6HHB SHOP, 6HHB/TOT, 6HTDTAL /
      DATA HOW/6HORIVER, 6HHWYPER, 6HALLPER/
      DATA CMD/1,2,3,3,2,2/, PURP/1,2,3,4,5,4,5,6,6/
      CO 30 L = 1,10
      DD 30 K = 1,3
      00 30 J = 1,2
      00 30 I = 1,241
3
      COUNT ([, J, K, L) = )
      CO 40 1=1,9
4
      HOUR ( [ ) = 0
      RFCORD = 0
      WRITE [6,12)
      00 36 1 = 1,240
      HH(1+1)=FLOAT(1)/10.
30
      CONTINUE
      HH(1)=0.0
      GO TO 1
      READ (8,110) MODE(15), PURPF(15), PURPP(15), PURPT(15), TIMES(15),
     1TIMEA(15)
      GO TO 69
1
      READ (8,10) [MODE([]), PURPF([]), PURPP([]), PURPT([]), TIMES([]), TIME
     1A(1), 1 = 1,15)
64
      RECORD = RECURD + 1
      00 31 N = 1.15
      1F (RECORD .EQ. 5064 .AND. N .LT. 15) GO TO 31
      TS=T[MES(N)+1
      TA=TIMEAIN)+1
      IF(TS.LT.241) GO TO 75
      TS=15-240
75
      IF(TA.LT.242) GO TO 75
      TA = TA - 240
7 +
      IF (TS .LT. 1 .OR. TA .LT. 2) GO TO 90
      (F (TS .GT. 240 .OR. TA .GT. 241) GO TO 90
      M = MODE(N)
      M = CMD(M)
      IF (M .LT. 1 .OR. M .GT. 3) GO TD 90
      PI = PURPF(N)
      P2 = PURPP(N)
      P3 = PURPT(N)
      IF (P1 .LT. 0 .OR. P1 .GT. 9) GD TO 90
      IF (P2 .LT. 0 .OR. P2 .GT. 9) GU TO 90
      IF (P3 .LT. 0 .OR. P3 .GT. 9) GO TO 90
      IF (P1 .NE. 0 .AND. P3 .NE. 0) GO TO 61
      IF (P1 .EQ. 0) GD TO 62
      IF (P1 .GT. 2) GD TD 64
      COUNT(TS, 1, M, P1+6) = COUNT(TS, I, M, P1+6) + 1
      COUNT(TA, 2, M, P1+6) = COUNT(TA, 2, M, P1+6) + 1
      GO TO 63
```

```
IF (P3 .EQ. 0) GO TO 90
60
      IF (P3 .GT. 2) GO TO 64
      COUNT(TS,1, M, P3+6) = COUNT(TS, 1, M, P3+6) + 1
      COUNT(TA, 2, M, P3+6) = COUNT(TA, 2, M, P3+6) + 1
      GU TO 63
      IF(M .NE. 1 .OR. (P1 .NE. 8 .AND. P3 .NE. 81) GO TO 63
64
      IF (P2 .NE. 1 .AND. P2 .NE. 2) GO TO 63
      COUNT(TS, 1, 1, P2+6) = COUNT(TS, 1, 1, P2+6) + 1
      COUNT(TA, 2, 1, P2+6) = CUUNT(TA, 2, 1, P2+6) + 1
      COUNT(TS, 1, M, 9) = COUNT(TS, 1, M, 9) + 1
63
      COUNT(TA, 2, M, 9) = COUNT(TA, 2, M, 9) + 1
      IF (P3 .EQ. 8 .AND. P1 .NE. 8 .AND. M .EQ. 1) P3 = P2
      IF(P1.EQ.O) P1=P3
      IF(P3.E0.0) P3=P1
      P1 = PURP(P1)
      P2 = PURP(P2)
      P3=PURP(P3)
      IF (P1-P3) 66,67,67
66
      COUNT(TS, 1, M, P1) = COUNT(TS, 1, M, P1) + 1
      COUNT(TA, 2, M, P1) = COUNT(TA, 2, M, P1) + 1
      CO TO 68
      COUNT(TS, 1, M, P3) = COUNT(TS, 1, M, P3) + 1
      COUNT(TA, 2, M, P3) = COUNT(TA, 2, M, P3) + 1
6 □
      COUNT(TS, 1, M, 10) = COUNT(TS, 1, M, 10) + 1
      COUNT(TA, 2, M, 10) = COUNT(TA, 2, M, 10) + 1
      GO TO 31
9
      WRITE (6.11) RECORD, N
31
      CUNTINUE
      IF(5063-RECORD) 71, 70, 1
71
      CUNTINUE
      00 35 L = 1.10
      CO 35 K = 1.3
      0 = XAM
      MIN=0
      IF(K.EG.3) GO TO 46
      CO 39 J=1,2
      CO 39 I=1,241
3+
      COUNT(I,J,K+1,L)=COUNT(I,J,K+1,L)+CCUNT(I,J,K,L)
46
      CONTINUE
      CO 44 [=2,232
      18=I+8
      HOUR ( 181=0
      C() 44 J=1.18
44
      HOUR(IA)=HOUR(IA)+COUNT(J,1,K,L)
      DO 32 J = 1,2
      CO 32 I = 1.240
32
      COUNT(I+1, J, K, L) = COUNT(I+1, J, K, L) + COUNT(I, J, K, L)
      IF(COUNT(241,1,K,L).NE.COUNT(241,2,K,L)) WRITE(6,16) TABLE(L),
     SHOW(K)
      SUM
              =COUNT(240,1,K,L)
      00 33 I = 1,240
      COUNT(I, 1, K, L) = COUNT(I, 1, K, L)-COUNT(I, 2, K, L)
33
      IF(COUNT(I,1,K,L).LT.MIN)
                                      MIN
                                              =COUNT(I,1,K,L)
      IF(MIN)
                   72,73,91
72
      CO 45 I=1.240
45
      COUNT(I,1,K,L)=COUNT(I,1,K,L)-MIN
73
      DO 41 I=10,240
      HOUR(I)=HOUR(I)+COUNT(I-9,1,K,L)
41
      IF(HOUR(I).GT.MAX) MAX=HOUR(I)
      WRITE(6,13) TABLE(L), HOW(K)
38
      00 \ 37 \ I1 = 1,40
```

```
I2 = I1 + 200
37
      WR1TE(6,14)
                          (HH(I+1), HOUR(I),
                                                       I = I1.12.43
      WRITE (6,15) SUM, MIN, MAX
   35 CONTINUE
      WRITE(6,12)
      READ (5,19) L1,L2,K1,K2
      CALL PLOTS(WORK(1),500,0)
      CALL SYMBOL (0.0,0.0,0.2,10H3362*SHUNK,90.0,10)
      CALL PLDT(1.0, 0.0, -3)
      DD 42 L=L1,L2
      UU 42 K=K1,K2
      CO 43 I=1,240
      Y([)=FLOAT(COUNT([,1,K,L))
43
      CALL SCALE(X,12.0,240,1,10.0)
      CALL SCALE(Y,10.0,240,1,10.0)
      CALL AX15(0.0,0.0,TITLE,-24,12.0,0.0,X(241),X(242),10.0)
      CALL AXIS(0.0,0.0,5HTRIPS,5,10.0,90.0,Y(241),Y(242),10.0)
      CALL SYMBOL (4.0, 10.0, J.3, T301, 0.0, 18)
      CALL SYMBOL(5.0, 9.5, 3.2, TABLE(L), 3.0,6)
      CALL SYMBOL(7.0, 9.5,0.2, HOW(K),0.0,6)
      WRITE(6,17)(Y(I),1=1,242)
      CALL LINE(X,Y, 240,1,1,5)
      IF(L.EU.10.AND.K.EQ.3) GO TO 42
      CALL PLOT(13.0,0.0,-3)
   42 CONTINUE
      CALL PLOTIO, 0, 9991
      STOP
91
      WRITE(6,18) TABLE(L), HOW(K)
      STOP
1
      FORMAT (15(41X, 411, 213, 33X))
110
      FURMAT (41X, 4I1, 2I3)
      FORMAT (25H INPUT DATA ERROR, RECORD, I5, 5H CARD, I3, 14H, CARD I
11
     IGNORED)
12
      FORMAT (19H1PEAK HUUR ANALYSIS)
      FORMAT(1H1, 35X, 51HIRTADS-PURDUE 1964 TRIP ACCUMULATIONS IN THE S
13
     1YSTEM// 45X, 5HTABLE, 1x, A6, 3X, 3H---, 3X, 4HMODE, 1X, A6////
     22x, 6(3x, 6HPERIOD, 2x, 5HTRIPS, 4x)/ 2x, 6(3x, 6H----, 2x, 5H-
     3 - - - - - - 4 \times 1 / / 1
      FORMAT (2X,6(4X,F4.1,3X,I5,4X))
14
   15 FURMAT (///37HOSUM DF 24 HOUR TRIPS IN THE CATEGORY,17/14H MINIMUM
     $ VALUE, 23X, 17/1X, 13HMAXIMUM VALUE, 23X, 17)
16
      FORMAT(15H CHECKSUM ERROR, 2(2X, 46))
      FURMAT (1H0,16F7.0/(1x,16F7.0))
17
      FORMAT (1X,19HMINIMUM VALUE ERROR,2(2X,A6))
1.8
19
      FORMAT (416)
      END
```

```
$IBFIC LENGTH SMAP, DECK
      REWIND 8
      REWIND 9
      INTEGER DIONE, DIONE, RECORD, DCARD, DCARD
      INTEGER CARD(17,15), EQUIV(790), TREE1(428), TREE2(428), TREE3(428)
      RECORD = 0
      MAX1 = 0
      MAX2= 0
      MAX 3= 0
      INDEX=C
      NERR = 0
      JPO = 1
      CALL SLITE(0)
      CALL SLITE(1)
      CALL ROBINITREEL(1), TREE2(1), TREE3(1), INDEX)
      READ (5,11) EQUIV
      ICK1 = 0
      START = TIME(DUM)/1000.
      ISTART = START/3600.
      WRITE 16,17) TSTART
1
      READ (8,10) CARD
60
      RECORD = RECORD + 1
      00 30 1 = 1,15
      IF (RECORD .EQ. 5094 .AND. 1 .GT. 1) GO TO 67
      CCARD = CARD(6.1)/10
      DCARD = CARD(8,I)/10
      DO 31 J = JPO, 789, 2
      JP0 = J
      IF ! OCARD -EQUIV(J)) 90,61,31
31
      CONTINUE
      GD TD 90
      CZDNE = EQUIV(J+1)
61
      IF (DZONE-INDEX) 30,64,63
63
      CALL RDBIN
      CALL SLITET(1, IEOF)
      1F ([EOF .EQ. 1) GO TO 91
      JPD=1
64
      IF (CARD(14,I)) 90,165, 90
165
      IF (CARD(13,1)) 90,265, 90
265
      IF (CARD(17,1)) 90, 65, 90
65
      CU 32 J = JPD, 789, 2
      JPD = J
      IF ( DCARD
                   -EQUIV(J)) 69,62,32
32
      CONTINUE
      GO TO 90
      DZONE = EQUIVIJ+1)
62
      CARD(14,1) = TREE1(DZONE+1)
      CARD[13,1] = TREE2(DZONE+1)
      CARD(17.I) = TREE3(DZONE+1)
      IF (CARD(14,1) .GT. MAX1)MAX1 = CARD(14,1)
      1F (CARD(13,1) .GT. MAX2) MAX2 = CARD(13,1)
      IF (CARD(17,1) .GT. MAX3)MAX3= CARD(17,1)
      GO TO 30
 69
      IF (CARD(6,1) .EQ. DCARD+10) GD TO 90
      JPD = 1
      GD TD 65
 90
      WRITE (6,12) RECORD , I, JPO, EQUIV(JPO), EQUIV(JPO+1), JPD,
     $ EQUIV(JPD), EQUIV(JPD+1), CZONE, DZDNE, INDEX
      WRITE (6,210)(CARD(N,I), N=1,17)
      NERR = NERR + 1
      IF (NERR .LT. 100) GO TO 30
```



```
WRITE (6,15)
      GO TO 68
3 ∴
      CONTINUE
      WRITE (9,10) CARD
67
      ICK = RECURD/100 - ICK1
      IF (ICK .LT. 1) GO TO 70
      PHASE = TIME(DUM)/1000. - START
      WRITE (6,16) RECORD, PHASE
      ICKI = RECORD/100
      START = TIME(DUM)/1000.
 70
      IF (RECORD-5093) 1, 2, 66
      READ (^{\circ},110) (CARD(I,1), I = 1,17)
      00 34 J = 2, 15
      00 33 I = 1.17
3 3
      CARD(I,J) = 0
      CARD(6,J) = 9999
      CARD(8,J) = 9999
34
      GO TO 60
      CALL SLITE(2)
65
      CALL ROBIN
      WRITE (6,13) MAX1, MAX2, MAX3
      GU TO 68
      WRITE [6,14] RECORD, I, INDEX
91
      CONTINUE
68
      END FILE 9
      KEWIND 9
      REWIND 8
      STOP
                (15(4A6, A5, I4, A4, I4, 4A6, 2I2, 2A6, [3])
 10
      FURMAT
                   (4Α6, Δ5, Ι4, Δ4, Ι4, 4Α6, 2Ι2, 2Α6, Ι3)
110
      FURMAT
      FURMAT [20X, 4A6, A5, I4, A4, I4, 4A6, 2I2, 2A6, I3]
210
      FORMAT (2(3X, I3))
11
      FORMAT (1x, 18HDATA OR SORT ERROR, 16, 14, 914)
12
      FORMAT(1X, 23HMAXIMUM TREE TIMES WHRE, 318)
1 3
14
      FORMAT (1X, 25HEND OF FILE ON TREE INPUT, 16, 14, 14)
15
      FURMAT (30H NUMBER OF ERRORS EXCEED LIMIT)
 16
      FURMAT [25HOPHASE TIME CHECK, RECORD, 15, 5H TOOK, F8.3,
     § 8H SECONDS I
      FORMAT(22H1PROCESSING STARTED AT, F7.3)
 17
      END
SIBMAP BINTR
               DECK
RUBIN SAVE
       SL T
               2
       TRA
               #+2
       TRA
               END
       SLT
               1
       TRA
               ROON
       CLA
                3,4
               TROL
       STA
       STA
               TR01+1
       CLA
               4,4
               TR02
       STA
       STA
               TR02+1
     ~ ČLA
               5,4
               TR03
       STA
       STA
               TRO3+1
       CLA
               6,4
       STA
               TR01+3
       STA
               TR02+3
       STA
               TR03+3
               .UPEN.4
       TSX
```

```
PZE
                 I REEO I
                 .OPEN,4
        TSX
        PZE
                 TREE02
                 .UPEN,4
        TSX
        PZE
                 TREE03
        TRA
                 BACK
                 .READ,4
REGN
        TSX
        PZE
                 TREED1,, ERR
        PZE
                 EOF,, ERR
TROI
        IURT
                 **,,**
        CLA
                 * *
                 18
        AKS
        STO
                 * *
        TSX
                 .READ,4
        PZE
                 TREED2,, ERR
        PZE
                 EOF,,ERR
TR02
        IORT
                 **,,**
        CLA
                 .
        AR S
                 18
        CAS
                 * *
        TRA
                 ERROR
        TRA
                 *+2
        TRA
                 ERROR
        TSX
                 .READ,4
        PZE
                 TREE03,, ERR
        PZE
                 EOF,, ERR
TR03
        IORT
                 **,,**
        CLA
                 * *
        ARS
                 18
        CAS
                 * *
        TRA
                 ERROR
        TRA
                 * + 2
        TRA
                 ERROR
BACK
        RETURN
                 RDBIN
ECF
        SLN
E VD
        TSX
                 .CLOSE,4
        PZE
                 TREE01
        TSX
                 .CLOSE,4
        PZE
                 TREE02
                 .CLOSE,4
        TSX
        PZE
                 TREEQ3
        TRA
                 BACK
ERROR
        TSX
                 .WRITE,4
        PZE
                 .UN06.,, ERR
        IORT
                 MESAG,,5
        TRA
                 E O F
                 SYSDMP
EЧR
        TRA
TREEDI FILE
                 ,UT5, BLK=428, LOW, BIN, HOLD, INPUT, MOUNT
TREE02 FILE
                 ,UT6,BLK=428,LOW,BIN,HOLD,INPUT,MUUNT
                 , JT7, BLK=428, LOW, BIN, HOLD, INPUT, MOUNT
TREED3 FILE
MESAG
        BCI
                 5, ERROR IN TREE CORRELATION
        END
```

```
$IBFTC RANDOM DECK, SMAP
      CUMMON/A/NUMBER(11500), ITEM
      COMMON/B/START, IRUN, ISTEP, PASSED
      EXTERNAL MOVE, COMPAR, KEYLOC
      REWIND 8
      TSTART = TIME(XD) / 3600000
      CSTART = DATE(XD)
      WRITE(6,11 ) DSTART, TSTART
      IRUN = 0
      SIZE = 86396.
61
      START = TIME(XD)/1000.
      IRUN = IRUN + 1
      WRITE(6, 12 ) IRUN, START
      ISTEP = 0
      SIZE = SIZE-10000.
      00 31 1 = 1,11500
      NUMBER(I) = IFIX(SIZE*FLRAN(XD1)
62
      IF(NUMBER(I)) 62,62,31
31
      CONTINUE
      CALL POST
      CALL QIKS(1,11500, MOVE, COMPAR, KEYLOC)
      CALL POST
      NRPT = 0
      00 32 I = 2.11500
      IF(NUMBER(I-1).NE.NUMBER(I)) GO TO 32
      NUMBER(I-1) = 0
      NRPT = NRPT+1
32
      CONTINUE
      CALL POST
      NELIM = 0
      NEXTRA = 1500-NRPT
      IF(NEXTRA) 91,164,63
      EXTRA = NEXTRA
63
      RANGE = (10000.+EXTRA)/EXTRA
      ISTART = RANGE *FLRAN(XD)
      ISTART = IABS(ISTART)
      WRITE (6,14) IRUN, ISTART, RANGE, NEXTRA
      II = ISTART - 1
      DO 132 I=1.10
      I1 = I1 + 1
      IF (NUMBER(II).EQ.O) GO TO 132
      NUMBER(II) = 0
      NELIM = 1
      GU TU 363
132
      CONTINUE
      WRITE (6,15) ISTART, IRUN
      GO TO 68
263
      CONTINUE
363
      H = ISTART
      DO 34 I=2, NEXTRA
      H = H + RANGE
      L = H + 0.5
      IF (NUMBER(L) .EQ. 0) GO TO 163
      NUMBER(L) = 0
      GO TO 64
163
      00 33 J = 1.9
      K5 = L + J
      IF(NUMBER(K5) .EQ.0) GO TO 33
      NUMBER(K5) = 0
      GO TO 64
33
      CONTINUE
```

```
WRITE(6, 15 ) I, IRUN
      GD TO 68
64
      NELIM = NETIM+1
34
      CONTINUE
      IF (NEL [M. EQ. NEXTRA) GU TO 164
      WRITE (6,16) IRUN, NEXTRA, NELIM
      CO TO 68
164
      CALL PUST
      J1 = 1
      NZERO = 0
      CO 35 [=1,11500
      1F(NUMBER(1).NE.O) GO TU (35,67),J1
      NZERO = NZERO + 1
      GO TO (65,66), J1
65
      J1 = 2
      K = 0
      K = K+1
66
      GO TO 35
 6.7
      K6 = I - K
      NUMBER(K6) = NUMBER(I)
35
      CONTINUE
      IF (NZERU .EQ. NELIM+VRPT) GD TO 167
      WRITE (6,17) IRUN, NZERO, NELIM, NRPT
      GO TO 58
1 - 7
      CALL POST
      WRITE (8,18) (NUMBER[[],[=1,10000)
      TCHICH = TIME(XD)/60000. - TSTART*60.
      PHASE = PASSED / 60.
      IF (TCHECK+PHASE .LE. 10.0) GO TO 267
      WRITE (6,23) IRUN, TCHECK
      IF (IRUN .LT. 5) GO TO 61
267
      WRITE (6,19) (NUMBER(I), I=1,10000)
      GO 10 69
      WRITE(6,13 ) IRUN, NEXTRA
      WRITE (6,20)
 6.9
      END FILE 8
      REWIND 8
      TEND = IIME(XD)/60000.
      ELAPSE = TEND+TSTART +60.
      WRITE (6.21) ELAPSE
      STOP
 11
      FORMAT (39HIRANDOM NUMBER GENERATION AND SURT, RUN, FR.1,
     $ 12H, START TIME, F7.31
      FURMAT (4H RUN, I2, 11H STARTED AT, F10.3)
 12
      FORMAT (22HOERROR AFTER SORT, RUN, [2, 14H EXTRA NUMBERS, [7]
 13
 14
      FORMAT (25HOAT DELETION PHASE OF RUN, 12, 13H START WAS AT, 13,
     $ 12H WITH GAP OF, F8.4, 10H TO DELETE, IS, 8H NUMBERS)
      FORMAT (33HOZERO SEQUENCE ERROR, STARTING AT, 14, 7H ON RUN, 121
 15
      FORMAT (29HOELIMINATION CHECK ERROR, RUN, 12, 8H, EXTRA=, 14,
 16
     $ 13H, ELIMINATED=, 14)
      FORMAT (23HOF1NAL CHECK ERROR, RUN, 12, 8H, ZEROS=, 14,
 17
     $ 13H, ELIMINATED=, 14, 10H, REPEATS=, 14)
 18
      FORMAT (100015)
      FORMAT (12H1NORMAL EXIT/(1X,2016))
 19
 20
      FORMAT (14HIABNORMAL EXIT/1H1)
 21
      FORMAT (20HOTIME SINCE START IS, F7.3)
      FORMAT (32HOTIME CONSTRAINT IS BINOING, RUN, 12,
 23
     $ 21H, TIME OF TERMINATION, F10.31
      ENO
```

```
$IBFTC SAMPLR SMAP, DECK
      COMMON /B/START, IRUN, ISTEP, PASSED
      REAL LENGTH(24,12,31
       INTEGER COUNT(24,12,3),T,P,PF,PP,PT,M,CMD(6),PURPI9), TS, TA
      CIMENSION INED(7,315), NUMBER(1000), IOUT(5), IN(5)
      DIMENSION TITLE(3), HEAD(12)
      DATA CMD/1,2,3,3,2,2/,PURP/1,2,3,4,5,4,5,6,6/
      DATA TITLE/6HDRIVER,6HPASNGR,6HTRNSIT/,
     * HEAD/4HWORK,4HSHOP,6HSCHOOL,5HPB-MD,5HSR-EM,5HOTHER,6HHBWORK,
     $ 6HHBSHOP,6HHB-SCL,6HHBPBMD,6HHBSREM,6HHBOTHR/
      DATA IOUT/3,2,3,2,0/, IN/0,3,2,3,2/
      REWIND 2
      REWIND 3
      REWIND 8
      REWIND 9
      IRUN = 0
      NNR = 0
      NERR = C
      NS = 0
      ISIZE = 76397
      NN = 1000
      NDRBI = 5094
      NCLRI = 1
6 I
      IRUN = IRUN+1
      START = TIME(DUM)/1000.
      WRITE(6, 11 ) IRUN, START
      ISTART = 1
      ISTEP = 100 * IRUN
      ISIZE = ISIZE - NS
      NUMBER(1000) = ISIZE
      NS = 0
      NCST = 0
      NC = 0
      NDR = 0
      NDRBO = 0
      CO 33 K=1.3
      DO 33 J=1,12
      CO 33 I=1,24
      LENGTH(I, J, K) = D.
      CCUNT(I, J, K)=0
 33
      CONTINUE
      NI = IN(IRUN)
      NO = IOUT(IRUN)
      IF (NERR .GT. 100) GO TD 94
261
      IF(NN.LT.1000) GO TO 62
      NMPREV = NUMBER (1000)
      READ(9, 12 ) NUMBER
      NNR = NNR+1
      NN = I
      IF (NUMBER(1)) 91,91,161
1 \in I
      NC = NMPREV - NUMBER(1) + NC
      CALL POST
      IF (NCST .EQ. 0) GO TO (140,78,78,78,78), IRUN
      GO TO 65
      NN = NN+1
62
      IF(NUMBER(NN)) 91,91,64
 64
      NC = NUMBER(NN-1) - NUMBER(NN) + NC
65
      IF(NC.GT.NCST) GO TO 73
      IF (NC .LT. 1 .OR. NC .GT. 315) GO TO 93
      NS = NS+1
      M = INFO(1.NC)
```

```
PF = I \setminus FO(2, NC)
      PP = INFO(3, NC)
      PT = INFO(4, NC)
      TS=INFO(5,NC)
      1A = INFO(6,NC)
      S = INFO(7.NC)
      M=CMD(M)
      IF (M.LT.1.0R.M.GT.3)GC TO 90
      IF (PF.FQ.8.AND.PP.NE.O) PF = PP
      IF (PT.5Q.8.AND.PP.NE.0) PT = PP
      [F (PF.NE.O.AND.PT.NE.O) GO TO 67
      IF (PF.EU.Q.AND.PT.EQ.O) GO TO 90
      P = MAXO(PF,PT)
      P = PURP(P)
      GO TO 68
 67
      PF = PURP(PF)
      PT = PURP(PT)
      P = MINO(PF,PT)
 68
      IF (P.LT.1.OR.P.GT.6) GU TO 90
      IF (TS.GT.TA) GD TO 71
      T =
              (TS+TA)/20
 69
      IF(T.LT.1.0R.T.GT.24) GD TO 90
      INFD(1,NC) = 0
      COUNT(T,P,M)=CBUNT(T,P,M)+1
      LENGTH(T,P,M) = LENGTH(T,P,M) + S
      IF(PF.NE.Q.AND.PT.NE.O) GO TO 70
      CUUNT(T,P+6,M) = COUNT(T,P+6,M) + 1
      LENGTH(T,P+6,M)=LENGTH(T,P+6,M) + S
      IF(NS.LT.10000) GD TO 261
70
      IF (NDR .NE. NDRBI) GO TO 92
      GO TO 72
 71
             (TS+TA+240)/20
      IF (T.GT.24) T= T-24
      GO TO 69
      00341 = 1,3
 72
      DO 34 J=1,12
      CO 34 K=1,24
 34
      LENGTH(K, J, I)=
                           LENGTH(K,J,I)/FLOAT(COUNT(K,J,I))
      CO 35 K=1,3
      WRITE(6,13) IRUN, TITLEIK), HEAD
      00 35 [=1,24
      wRITE(6,14) I, (COUNT(I,J,K), J=1,12)
      PUNCH 18, (COUNT(I, J, K), J = 1,12), K, I
 35
      CONTINUE
      DO 36 K=1.3
      WRITE (6,15) IRUN, TITLE (K), HEAD
      CO 36 I=1,24
      WRITE(6,16) I, (LENGTH(I,J,K), J=1,12)
      PUNCH 19, (LENGTH(I,J,K),J\pm1,12),K,I
      CONTINUE
 36
      GO TO 173
66
      END FILE NO
      REWIND 2
      REWIND 3
      WRITE (6,24) IRUN, NDRBO, NCLRO, NS, NDRBI, NCLRI
      ISTEP = 49 + 100 * IRUN
      CALL POST
      NDRBI = NORBO
      NCLRI = NCLRO
      IF (IRUN .LT. 4) GO TO 61
      REWIND 8
```



```
REWIND 9
      STOP
      IF (IRUN .LT. 4) GO TO 173
 73
      LEFT = 0
      NCST = 0
      NC = NC - 315
      GO TO 78
173
      N7 = 0
      JI = 1
      CO 38 I=1.NCST
      IF(INFO(1,1) .NE.0) GO TO (38, 76),J1
      GO TO (74,75), J1
74
      J1 = 2
      K = 0
75
      K = K+1
      NZ = NZ + 1
      GO TO 38
      KK = [-K
76
      00 37 J=1,7
      INFO(J,KK) = INFO(J,I)
3 7
      CONTINUE
35
      CONTINUE
      REM = NCST - NZ
      ITUP=REM-34.
      IF (ITOP .LT.35) GO TU 176
      CO 39 L=1,1TOP,35
      KK = L + 34
      WRITE (NO)
                    ((INFO(I,J), I=1,7), J=L,KK)
      NDRBO = NDRBO+1
39
      CUNTINUE
176
      LEFT = AMOD(REM, 35.)
      IF (NS .LT. 10000) GO TO 139
      K = KK + 1
      KK = KK + LEFT
      WRITE (NO) ((INFO(I,J), I=1,7), J=K,KK)
      NDRBO = NDRBO + 1
      NCLRO = LEFT
      €0 TO 66
      NC = NC - NCST + LEFT
139
      NCST = LEFT
      CO 40 J=1, LEFT
      JJ=J+KK
      00 40 I=1.7
      INFO(I,J) = INFO(I,JJ)
4.1
      CONTINUE
      ISTART=LEFT+1
      IF! IRUN.GT.1) GO TO 78
140
      CO 41 I=ISTART, 301, 15
      IF INDR .EQ. NDRBI) GO TO 141
      IFINDR . EQ. 50931 GO TO 77
      11 = 1 + 14
      READ(8, 21 ) ((INFO(J,K), J=1,7), K=I,I1)
      NCST = NCST+15
      NDR = NDR + 1
41
      CONTINUE
      GO TO 65
      IF (NC .GT. NCST) GO TO 93
141
      GD TO 65
77
      READ(8,121 ) (INFO(J,I), J=1,7)
      NCST=NCST+1
      NDR = 50 94
```

```
REWIND R
      GO TO 65
73
      CO 42 I=ISTART, 281, 35
      IF(NDR.EQ.NDRBI) GU TO 142
      I1 = I + 34
      IF (NDR .LT. NDRBI-1) GO TO 79
      NCST = NCST - 35 + NCLRI
      I1 = I + NCLR1 - 1
 19
      READ(NI) ((INFO(J,K), J=1,7), K=I,I1)
      NCST = NCST + 35
      NDR = NDR + 1
42
      CONTINUE
      GU TO 65
142
      IF (NC .GT. NCST) GO TO 93
      GO TO 65
 90
      NREC = ISIZE - NUMBER(NN)
      WRITE (6,17)
                   NREC,
                               (INFO(LL,NC),LL=1,7), M, P, T
      NERR = NERR + 1
      GO TO 261
91
      WRITE(6, 20 ) IRUN, NN, NUMBER(NN)
      NERR = NERR + 1
      GO TO 261
 42
      WRITE (6,23) IRUN, NDR, NDRBI
      NERR = NERR + 1
      GO TO 72
      WRITE (6,22) IRUN, NC, NCST, NN, NUMBER(NN)
 13
      NERR = NERR + 1
      GU TO 72
      WRITE (6,25) 1RUN, NN
      REWIND 2
      REWIND 3
      REWIND 8
      REWIND 9
      STOP
 10
      FORMAT (1H1)
      FORMAT (4H RUN, 12, 11H STARTED AT, F10.3)
 11
 12
      FORMAT (100915)
      FORMAT (1H1,39X,22HTRIP COUNTS FOR SAMPLE,12,9H BY MODE ,A6///
 13
     6 6X,4HHOUR,6X,12(3X,46)/6X,4H----,6X,12(3X,6H-----)//)
 14
      FURMAT(6X, I3, 6X, 12(3X, I6))
 15
      FORMAT(1H1,39x,31HAVERAGE TRIP LENGTHS FOR SAMPLE,12,9H BY MODE,
     $ A6////6X,4HHDUR,1X,12(3x,A6)/6X,4H----, 1X, 12(3x,6H-----)//)
 16
      FORMAT (6X, 13, 1X, 12(3X, F6.1))
      FURMAT (22H CARD IN ERROR, RECORD, 816, 318)
 17
      FORMAT(1216, 2HCT, 213)
 18
 19
      FORMAT (12F6.1, 3HLEN, 12, 13)
      FORMAT (25HOBAD RANDOM NUMBER ON RUN. I2. 5H. NN=. I6.
 20
     $ 11H, NUMBER IS, 16)
      FORMAT (15(41x, 411, 213, 18X,
 21
                                        I2, 13XII
121
      FORMAT
                (41X, 4I1, 2I3, 18X,
                                        I2, 13X)
      FORMAT (24HDDATA SEEKING ERROR, RUN, 12, 14H, CARD SOUGHT=, 14,
 22
     $ 15H, CARDS STORED=, I4, 5H, NN=, I5, 9H, NUMBER= I5)
      FORMAT (27HORUN TERMINATION ERROR, RUN, I2, 15H, RECORDS READ=,
 23
     $ I4, 24H, RECORDS ON INPUT TAPE=, I4)
      FORMAT (4H1RUN, 12, 9H PRODUCED, 15, 18H DATA RECORDS WITH, 14,
     $ 22H CARDS IN LAST RECORD,, I6, 14H SAMPLES, FROM, I5,
     $ 14H INPUT RECORDS, I3, 14H CARDS OVERLAP)
      FORMAT (49H1NUMBER OF ERRORS EXCEEDS TOLERABLE LIMIT, AT RUN, I2,
     $ 8H, NUMBER, [5]
      END
```

VITA

VITA

Gordon Arthur Shunk was born December 23, 1938, in Kankakee, Illinois. He attended elementary schools there and was graduated from Kankakee High School in June, 1956.

He attended DePauw University from September, 1956, until June, 1959, majoring in pre-engineering sciences and mathematics. The A.B. degree was awarded in June, 1961.

He transferred to Purdue University in June, 1959, and was awarded the B.S.C.E. degree in June, 1961, and the M.S.C.E. degree in June, 1962, until January, 1964, he was employed as a Highway Engineer (Trainee) by the U.S. Bureau of Public Roads. He returned to Purdue in February, 1964, to work on the Ph.D. and has been employed as a Graduate Instructor in Research since that time.

He is a Student Member of the Institute of Traffic Engineers, an Associate Member of the American Society of Civil Engineers, and an Academic Supporting member of the Highway Research Board. He is a member of Chi Epsilon and Tau Beta Pi and an Associate Member of the Society of the Sigma Xi.

He is married and has one son.

